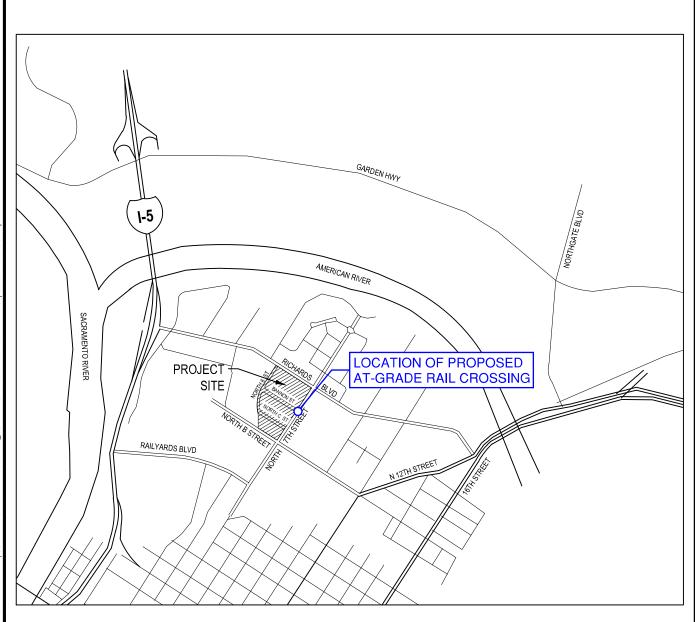


# Attachment A

Vicinity Map

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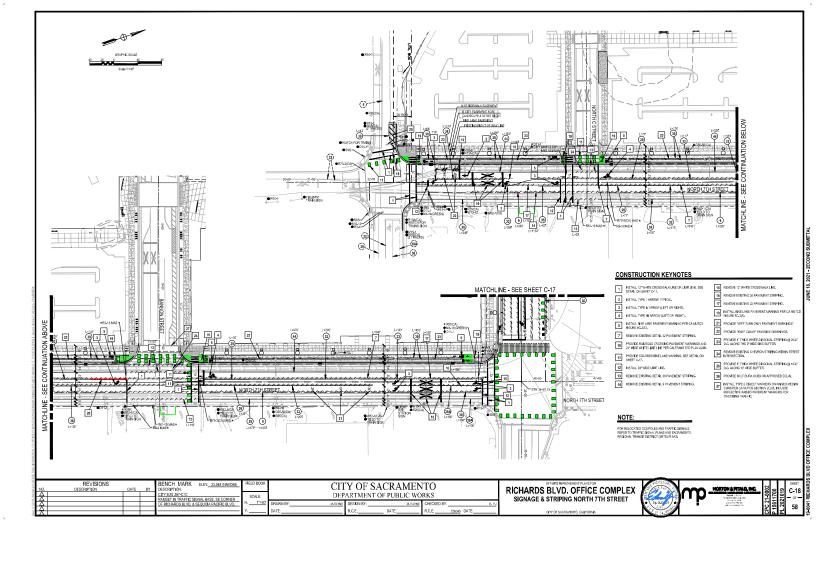
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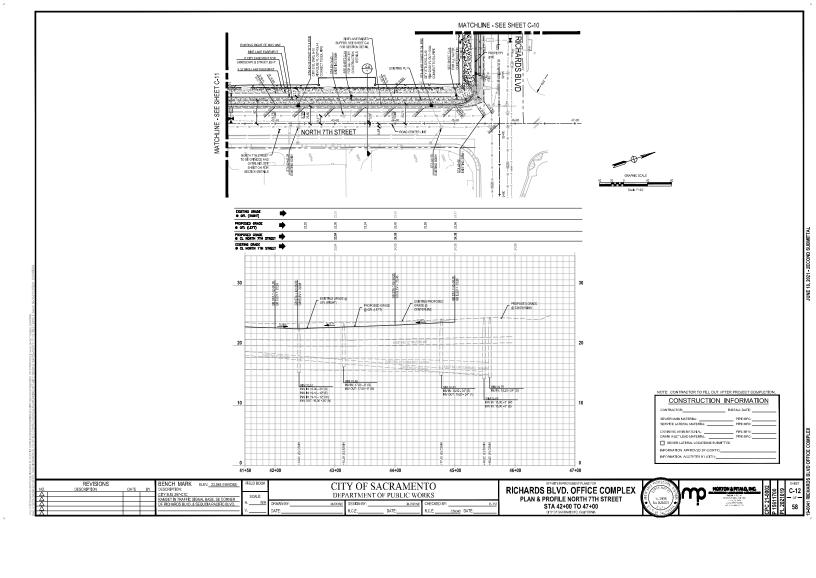
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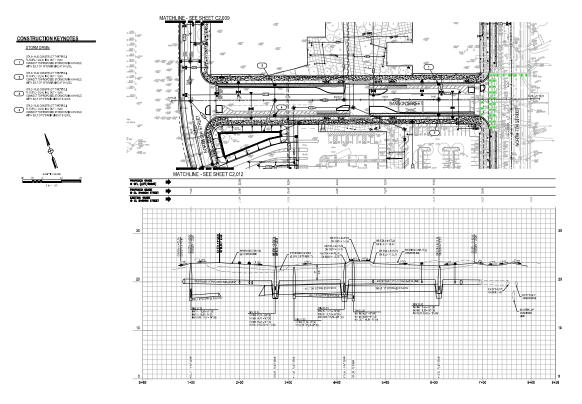
# Attachment B Proposed At-Grade Crossing Signing and Striping Plans



# Attachment C

Proposed At-Grade Crossing Grade Lines





HENSEL PHELPS

ZGF Decyfuse+Blackford

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ONNER CONTROL SE COLOR
ON TO STREET CO. SE COLOR
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CIVIL WORTON & PITALO, INC. 80) COOLIDGE DRIVE, SUITE 140 FOLSON, CA 19890

Insured No. Complex Co

RICHARDS BOULEVARD OFFICE COMPLEX 651 BANNON STREET 201 NORTH 6TH STREET SACRAMENTO, CA 95811

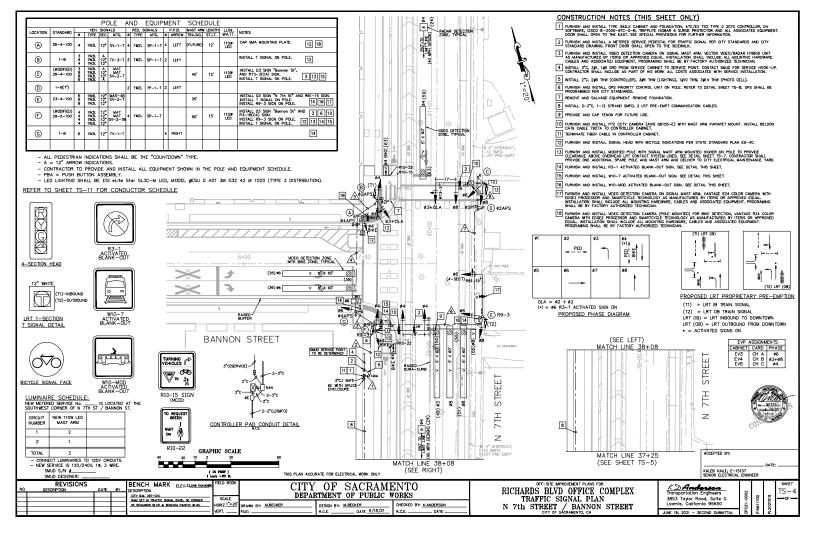
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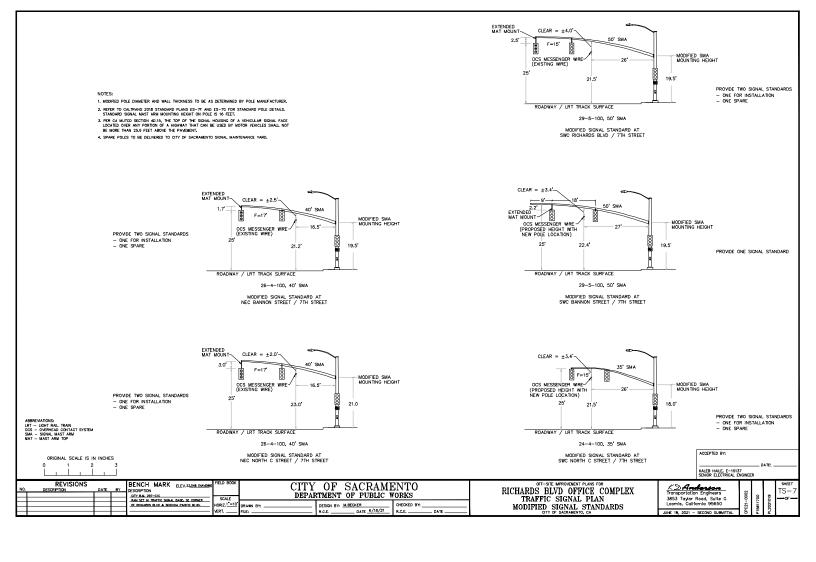


C2.011

# Attachment D

Proposed Intersection Signal Plans





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OFF-SITE IMPROVEMENT PLANS FOR
RICHARDS BLVD OFFICE COMPLEX
TRAFFIC SIGNAL PLAN
CONDUCTOR SCHEDULES
CITY OF SACRAMENTO, CA

THIS PLAN ACCURATE FOR ELECTRICAL WORK ONLY



KALES HAILE, E-19337
SONOR ELECTRICAL ENGINEER

SHEET
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ACCEPTED BY:

DESCRIPTION DATE BY DE	BENCH MARK ELEV, 23,048 (NAVDBS) FIELD BOOK ESCRIPTION OF THE BOOK SCALE	CITY OF SACRAM DEPARTMENT OF PUBLIC	ENTO Works
	OF RICHARDS BLVD & SEQUOIA PACIFIC BLVD. HORIZ,1"=20"	DESIGN BY: M.BECKER	CHECKED BY: K.ANDERSON R.C.E. DATE

# Attachment E

RBOC Final Traffic Operations Study

# **MEMO**

To: John Webber, DGS; Wendy Andreotti, HP; Jamie Lynne Athenour, HP; Anthony Bruzzone, ARUP

From: Ken Anderson, KD Anderson & Associates, Inc.

Date: November 3, 2020

**Re:** RBOC: Final Traffic Operations Study for Traffic Signal DCR's

### **Traffic Operational Analysis for RBOC DCR's**

### Approach

This revised traffic analysis follows our April 30, 2020 report and addresses comments received from the City of Sacramento. This analysis makes use of SimTraffic simulation to evaluate the operation of existing and proposed traffic signals in the area of the Richards Boulevard Office Complex (RBOC) as required by the City of Sacramento to support Design Concept Reports (DCR) for new traffic signal and traffic signal modifications. The RBOC is located at the southwest corner of Richards Blvd / N. 7<sup>th</sup> Street as noted in Figure 1 (attached). Traffic volume forecasts created for the RBOC EIR and subsequently supplemented for the Department of General Services (DGS) were adjusted to account for the quantity of employment and on-site parking now proposed, as well as the location of site access, parking and drop-off /loading. SimTraffic networks created for the RBOC EIR and subsequent document were then adjusted to account for the current access and internal circulation proposal. Resulting traffic operations expressed in terms of intersection delays and peak turn lane queues were projected and used to evaluate the feasibility of proposed access and internal circulation.

### **KEY CONCLUSIONS**

- 1. The three new signals proposed with the RBOC project are expected to operate with Levels of Service that satisfy the City of Sacramento's minimum LOS E standard in this area. However, conditions may be poorer during peak periods due to the effects of queuing from other intersections.
- 2. The overall flow of peak hour traffic in the area around RBOC is characterized by long queue of through traffic that extend through adjoining intersections. Under near term conditions the p.m. peak hour queue of westbound traffic on Richards Blvd is projected to extend back from the N. 5<sup>th</sup> Street intersection through N. 6<sup>th</sup> Street to N. 7<sup>th</sup> Street. In the eastbound direction, the through traffic queue created at N. 7<sup>th</sup> Street is expected to extend back through N. 6<sup>th</sup> Street to N. 5<sup>th</sup> Street in both the a.m. and p.m. peak hour. On N. 7<sup>th</sup> Street long queues are caused by the operation of the intersections south of RBOC. The southbound queues created at the Railyards Blvd and B Street intersections extend north through the two RBOC intersections, and in the evening that queue is projected to reach Richards Blvd. Long queues in other directions are found at locations south of the project site, but those queues do not directly affect the flow of traffic in the area of RBOC.

November 3, 2020

Page 2

It is important to note, however, that planned improvements along N. 7<sup>th</sup> Street that will accompany other approved development projects will improve traffic flow south of the RBOC area and reduce delays and queue lengths.

- 3. Left turn lane queues were evaluated from the standpoint of the design of individual intersections. In the eastbound direction the 95<sup>th</sup> percentile left turn queues on Richards Blvd at the N. 7<sup>th</sup> Street intersection is projected to would fill the left lane and make use of the Two-Way Left Turn area between the N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street intersections (refer to Table 6). However, visual review of the SimTraffic simulation reveals that eastbound through traffic is not impeded by queuing in the left turn lane.
- 4. Peak period 95<sup>th</sup> percentile left turn lane queues on N. 7<sup>th</sup> Street are projected to fill the area between the C Street and B Street intersections in the a.m. peak hour under Cumulative plus Project conditions (refer to Table 9). Review of simulation runs reveals that through traffic on N. 7<sup>th</sup> Street is sometimes blocked by spillover queues.
- 5. Earlier analysis suggested that in the short term left turn queueing in this area of N. 7<sup>th</sup> Street would be benefited by completion of other improvements that are not a part of the RBOC project but are expected to be completed by others (i.e., second southbound through lane on N. 7<sup>th</sup> Street south of B Street).
- 6. Preliminary alternatives evaluated at the N. 7<sup>th</sup> Street / C Street intersection included side street stop sign control on C Street and a northbound left turn prohibition with a traffic signal. However, those alternatives have been rejected due to the need for a traffic signal to accommodate the volume of traffic across the LRT tracks and due to the absence of alternative access to the RBOC in the near term.
- 7. Design decisions regarding access at N. 7<sup>th</sup> Street / C Street should consider the opportunities for alternative access that may be absent today but will be available in the future, as well as the relative effect on the RBOC project. In the short term eliminating northbound left turns at C Street would add traffic onto Bannon Street and could increase conflicts with the drop-off and loading functions on that street. However, in the long term the anticipated extensions of 5<sup>th</sup> and 6<sup>th</sup> Streets to Railyards Blvd would compensate for the absence of northbound left turns at C Street, if that prohibition proved to be necessary in the future.
- 8. The effects of internal RBOC circulation on the design of new traffic signals has been evaluated. While not related to the design of the N. 7<sup>th</sup> Street / C Street intersection itself, the queue of southbound through traffic on N. 7<sup>th</sup> Street caused by the operation of downstream traffic signals is projected to result in eastbound C Street queues that extend to the parking structure entrance. While not evident from simulation, uncontrolled pedestrian activity at the Bannon Street pedestrian crossing could result in peak period westbound queues that extend to the N. 7<sup>th</sup> Street signal.

## **BACKGROUND INFORMATION**

### **Project Description**

As noted in Figure 2 (attached), the RBOC proposes signalized vehicular and pedestrian access at the following locations which would be the subject of Design Concept Reports (DCR's):



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- New traffic signal at Richards Blvd / N. 6<sup>th</sup> Street
- Traffic signal modification at Richards Blvd / N. 7<sup>th</sup> Street
- New traffic signal at N. 7<sup>th</sup> Street / Bannon Street
- New traffic signal at N. 7<sup>th</sup> Street / C Street
- Minor traffic signal modification at N. 7<sup>th</sup> Street / B Street

The RBOC includes a 1,294 space parking garage with access to C Street between N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street and to N. 6<sup>th</sup> Street between Richards Blvd and Bannon Street, as well as surface parking for visitors and employees with access to C Street. The RBOC includes designated drop-off and loading areas on Bannon Street as well as a pedestrian crossing on Bannon Street linking RBOC's office towers with the parking structure. No direct access to the project's parking areas is planned via Bannon Street.

### Traffic Volume Forecasts

**Resources.** The approach to preparing these traffic volume forecasts makes use of information developed for the RBOC DEIR, including the DEIR Transportation and Circulation Element (TCE) and DEIR Appendix G Transportation Data (GTD), as well as supplemental data provided by Fehr and Peers (F&P) for DGS in September 2019. The TCE identifies baseline plus project and cumulative plus project a.m. and p.m. peak hour traffic volume data for 15 locations, including generalized RBOC access points on 7<sup>th</sup> Street and on Richards Blvd. The TCE also identifies the regional trip distribution assumptions made for the project. GTD data includes assumptions relating to RBOC employee travel mode and schedule which supplement the DEIR's trip generation estimate. The supplemental F&P data addressed a revised project occupancy load, and its forecasts also reflect refined assumptions for site access and circulation. Those assumptions included the extension of Bannon Street westerly to 6<sup>th</sup> Street as a drop-off / loading area, the extension of C Street west of 7<sup>th</sup> Street and the extension of 6<sup>th</sup> Street south through Bannon Street to C Street. These assumptions for circulation are consistent with the RBOC project now proposed.

Assumptions for Baseline Plus Project Forecasts. The approach taken to create traffic volume forecasts for this analysis makes use of the work done by F&P as the starting point and accounts for RBOC project changes in comparison to the F&P assumptions. Table 1 compares the assumptions and trip generation forecasts made under each resource.

The general on-site employment will be slightly more than was noted in the F&P work under baseline conditions but will ultimately reach 5,000 persons. Because the difference between the F&P assumption and the current opening day employment is very small (i.e., 105 employees), the RBOC trip generation forecasts is initially based on F&P opening day employment, but the cumulative (i.e., 20 year) trip generation is assumed to be proportionately greater in the future when 5,000 employees may be on the site.

The project now proposed will contain 210 more parking spaces than assumed in the TCE or by F&P. The peak hour traffic associated with these spaces has been estimated based on the share of all employee commute trips occurring in the a.m. peak hour (47.5%) and the p.m. peak hour (62.3%) noted in the GTD. A total of 100 a.m. and 130 p.m. trips are forecast. These additional trips have been assumed to increase the volume in or out of the site at the Richards Blvd /  $6^{th}$  Street and N.  $7^{th}$  Street / C Street intersections in proportion to the volumes identified by F&P.



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		TABLE 1 ONS / TRIP GENERATION	
Description	DEIR TCE	F&P 9/2019	Current RBOC
Parking spaces	1,420	1,420	1,630
Building size	1,375 ksf	1,375 ksf	1,250 ksf
Employees	6,000	4,545 <sup>1</sup>	4,650 opening day 5,000 ultimate
Richards Blvd Access	Roughly 6th Street	6 <sup>th</sup> Street	6 <sup>th</sup> Street
7 <sup>th</sup> Street Access	Roughly Bannon Street	at Bannon Street to support drop-off and loading activities at C Street	at Bannon Street to support drop-off and loading activities at C Street
AM Trip Generation	1,416 in / 412 out	1,154 in / 191 out	1,154 in / 191 out 1,269 in / 210 out 20 yr
PM Trip Generation	611 in / 1,957 out	310 in / 1,600 out	310 in / 1,600 out 331 in / 1,760 out 20 yr
Sum of AM driveways	988 in / 420 out	1,001 in / 579 out	1,101 in / 579 out <sup>2</sup>
Sum of PM driveways	617 in / 1,352 out	315 in / 1,059 out	315 in / 1,189 out <sup>3</sup>
Sum of AM driveways 20 yr	1,120 in / 475 out		1,120 in / 475 out
Sum of PM driveways 20 yr	650 in / 1,080 out		650 in / 1,080 out

<sup>&</sup>lt;sup>1</sup> represents number of employees present on typical midweek day considering those that telecommute, work part time or work from field offices.

As noted in the DEIR TCE, the presence of additional parking would have the effect of diverting some traffic by employees who would otherwise park elsewhere or use a ride hailing service. Thus, the presence of additional parking would have no effect on the amount of a.m. traffic into the area or outbound p.m. peak hour traffic on a regional basis. By eliminating the second leg of a ride hailing trip, the overall volume of project traffic could be reduced.

The following steps were taken to create the Baseline plus Project peak hour forecasts for this analysis.

- 1. Started with F&P September a.m. and p.m. peak hour forecasts.
- 2. Add trips specifically related to the additional garage space to the F&P forecasts for site access and to the intersections within two blocks of the site.
- 3. To present a conservative assessment, assume no reduction in traffic anywhere due to elimination of the second leg of some ride-hail trips.
- 4. Distribute identified traffic volumes to and from on-site parking garage and parking lot access and through drop-off and loadings areas.

**Baseline Plus Project Peak Hour Traffic Volume Forecasts**. Figure 3 (attached) illustrates study locations. Figure 4 (attached) illustrates the a.m. and p.m. peak hour traffic volume forecasts used in this analysis.



<sup>&</sup>lt;sup>2</sup> assumes 100 more inbound trips based on 210 additional spaces with 47.5% filling in the a.m. peak hour

<sup>&</sup>lt;sup>3</sup> assumes 130 more outbound trips based on 210 additional spaces and 62.3% existing in the p.m. peak hour

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**Assumptions for Cumulative plus Project Forecasts.** The DEIR TCE presents Cumulative plus Project volumes which yield driveway trip totals that were somewhat different from the baseline forecasts. Among other reasons, the TCE attributed trip generation changes to the presence of additional parking in the Riverfront area and a corresponding decrease in RBOC drop-off activity. For this analysis the following steps were taken to create the Cumulative plus Project volumes to be used for simulation analysis.

- 1. Started with TCE traffic volume forecasts.
- 2. Identified the difference in turning movement volumes under Cumulative no Project and Cumulative plus Project conditions.
- 3. Adjusted that difference based on the ratio of the ultimate employment now anticipated (i.e., 5,000) and the TCE ultimate employment estimate (i.e., 5,000 / 6,000 or 17% reduction).
- 4. Added the adjusted difference to the Cumulative no Project volumes at study intersections.

For RBOC access points it was necessary to consider adjusting the TCE volumes to account for reduced site employment as well as increased on-site parking. The change in employment (17% reduction) was assumed to be generally balanced by the change in onsite parking (20% increase), and the TCE driveways total were assumed to remain applicable.

However, because the TCE assumed only one access on N. 7<sup>th</sup> Street, it was necessary to manually adjust the TCE's Richards Blvd driveway volume forecasts to account for the presence of the N. 7<sup>th</sup> Street / C Street access. This was accomplished by dividing the volumes between Bannon Street and C Street in general proportion to the driveway use identified under Baseline conditions. Resulting traffic volumes were subsequently distributed to on-site parking and drop-off loading areas.

**Cumulative Plus Project Peak Hour Traffic Volume Forecasts**. Figure 5 (attached) illustrates the a.m. and p.m. peak hour traffic Cumulative plus Project volume forecasts proposed for use in this analysis.

### **SIMULATION**

### Simulation Assumptions

**Source.** SimTraffic simulation networks were created for the RBOC TCE and subsequent study and were made available for this analysis. Those networks covered that area addressed in the traffic volume forecasts noted earlier and included Richards Blvd from Interstate 5 to N. 16<sup>th</sup> Street and N. 7<sup>th</sup> Street from Richards Blvd to Railyards Avenue. The existing light rail crossings on N. 7<sup>th</sup> Street at Richards Blvd and at B Street were included in the networks. Existing and Cumulative plus Project a.m./p.m. DEIR models were employed. In addition, Baseline plus RBOC models created for DGS in September 2019 were also provided and employed.

**Adjustments.** In addition to incorporating the traffic volumes noted above, the simulation models were modified to reflect the currently proposed intersection geometry at each intersection, the presence of two new Light Rail Crossings on N. 7<sup>th</sup> Street and the location of on-site facilities. The models were also modified to reflect pending light rail schedule changes (i.e., 15-minute headways in each direction).

Decisions were made with regards to other improvements that may be made outside of the scope of work included in the RBOC project, particularly at the N. 7<sup>th</sup> Street / B Street. While the RBOC project will improve the NW corner of the intersection, subsequent improvements to the SW corner and on N. 7<sup>th</sup> Street to the south will be made by other pending development. These improvements are assumed in the



cumulative condition, and the use of mixed-use transit-automobile lanes on southbound N. 7<sup>th</sup> Street in the area north of B Street was confirmed in consultation with City of Sacramento staff. The baseline condition assumes the SW corner improvements are not made.

The simulation models reflect applicable pedestrian activity. On N. 7<sup>th</sup> Street a pedestrian crossing is planned at the N. 7<sup>th</sup> Street / Bannon Street intersection, but no crossing on N. 7<sup>th</sup> Street is planned at the C Street crossing. An east side pedestrian crossing of Richards Blvd is planned at the N. 6<sup>th</sup> Street / Richards Blvd intersection. This location was assumed to share the appreciable pedestrian traffic between RBOC and the Township 9 LRT station with the N. 7<sup>th</sup> Street intersection. Internally, pedestrian activity at the Bannon Street crossing commensurate with parking garage use and employee schedule distribution (i.e., 1,200± pedestrians per hour) was input to the model.

The simulation model considers bicycle access issues at the N. 7<sup>th</sup> Street / Richards Blvd intersection created by the two-way cycle track on the west side of Richards Blvd. That facility ends on the southwest corner of the intersection, and northbound bicyclists may wish to turn left across the LRT tracks to head west or may need to cross the intersection to continue north. While preliminary alternatives consider whether these movements might be facilitated by an exclusive bicycle phase which holds automobile traffic, exiting northbound bicyclists will be mixed with pedestrians across Richards Blvd.

The simulation models were adjusted to reflect use of the Bannon Street pedestrian crossing and drop-off loading areas. The Bannon Street pedestrian crossing does not include active vehicular traffic controls but the crossing itself has been narrowed and accompanied by raised curbs. In this case motor vehicles would legally have the right of way through the crossing but are not to enter the crossing when already occupied by pedestrians. This location was represented in the model as a low speed (i.e., 10 mph) all-way stop controlled intersection to reflect the interaction between pedestrians and vehicular traffic on Bannon Street using drop-off/loading areas.

**Traffic Signal Operational Assumptions** The simulation models reflect proposed traffic signal coordination assumptions. Under both Baseline plus Project and Cumulative plus Project Conditions existing coordination on Richards Blvd from N. 5<sup>th</sup> Street westerly and from N. 10<sup>th</sup> Street easterly was assumed to continue.

Under near term conditions the N. 7<sup>th</sup> Street / Richards Blvd intersection continues to be isolated. On Richards Blvd the operation of the N. 6<sup>th</sup> Street traffic signals was coordinated in both directions with the signal to the west. On N. 7<sup>th</sup> Street the signals were coordinated in the southbound direction at B Street and in the northbound and southbound directions at Bannon Street and C Street.

Slightly different coordination was used under Cumulative conditions. The N. 6<sup>th</sup> Street signals on Richards Blvd were coordinated in both directions. On N. 7<sup>th</sup> Street the signals were coordinated in both northbound and southbound directions from Bannon Street to B Street in the a.m. peak hour. In the p.m. the signals were coordinated in the southbound direction with northbound and southbound coordination at C Street and Bannon Street.

The simulation models included a 10-minute seeding period and a 15-minute simulation period. Ten runs were made, the outlying high and low runs were eliminated, and the average of the eight middle runs has been reported.

Microsimulation reflects the effects of random traffic flow variation, and in the case of locations adjoining light rail crossings, the random timing of light rail trains. Light rail trains can disrupt the flow of



background traffic and cause congestion that generally dissipates over the course of several traffic signal cycles. The overall effects of light rail activity on average intersection delay and queueing reported as a byproduct of simulation can vary greatly from run to run and depending on the point in the simulation run when the train arrives.

### Simulation Results

Simulation results are attached and include intersection delay and 50<sup>th</sup> and 95<sup>th</sup> percentile vehicle queue information. The simulations addressed all intersections included in the RBOC EIR analysis, and the results are included in the appendix.

**Levels of Service.** Table 2 presents Baseline plus RBOC Levels of Service at the three access locations and at four adjoining intersections. Table 3 presents similar information for Cumulative plus Project conditions. These results were compared to City of Sacramento standards as outlined in policy M1.2.2. That policy notes that LOS E is acceptable for this project because it is within ½ mile of a light rail station.

Review of Levels of Service results reveals that poor conditions are projected at off-site locations on N. 7<sup>th</sup> Street south of the project, and these conditions at may affect the operation of RBOC intersections. LOS F conditions with very long delays are forecast at the N. 7<sup>th</sup> Street / B Street and N. 7<sup>th</sup> Street / Railyards Blvd intersections. These conditions are primarily the result of the limited capacity at these locations. While planned improvements that are not a part of the RBOC would add travel lanes, increase the capacity of these intersections and improve the Level of Service, until those improvements are installed the operation of these intersections effectively meters the flow of northbound 7<sup>th</sup> Street traffic into the area of RBOC but also causes appreciable delays for southbound traffic that are not reflected in the intersection delay reports at upstream intersections. Thus, conditions at RBOC intersection on N. 7<sup>th</sup> Street may be worse than projected during peak periods.

**Peak Period Queues.** Table 4 presents the Average and 95<sup>th</sup> percentile queue length forecasts for left turn lanes and for other lanes at intersections in the vicinity of RBOC. Table 5 presents similar information under Cumulative plus RBOC conditions. Queue tables also present the length of current and proposed left turn lane lengths unless otherwise noted, and queues longer than storage are in **Bold.** The forecast queue are also illustrated in Figures 6-14.

The overall flow of traffic through the area of RBOC can be characterized based on the presence of through queues that extend back through adjoining intersections. For example, in the p.m. peak hour the queue of westbound traffic on Richards Blvd is projected to extend back from the N. 5<sup>th</sup> Street intersection through N. 6<sup>th</sup> Street to N. 7<sup>th</sup> Street. In the eastbound direction, the through traffic queues created at N. 7<sup>th</sup> Street extend back through N. 6<sup>th</sup> Street and to N. 5<sup>th</sup> Street in both the a.m. and p.m. peak hour. On N. 7<sup>th</sup> Street through traffic queues are caused by the operation of the intersections south of RBOC. The southbound queues created at the Railyards Blvd and B Street intersections extend north through the two RBOC intersections, and in the evening the southbound queue is projected to reach Richards Blvd. Long queues are found in other directions at the locations south of the project site, but these queues do not directly affect the flow of traffic in the area of RBOC.



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	BASELINE PLUS		BLE 2 PEAK HOUR L	EVELS OF	SERVICE	
			Baseline Plu	ıs Project (		TT
#	Intersection	A	M Peak Hour		PM Peak	Hour
		Control	Ave Delay (sec/veh)	LOS	Ave Delay (sec/veh)	LOS
6	N. 5 <sup>th</sup> St / Richards Blvd	Signal	18	В	15	В
7	N. 7 <sup>th</sup> St / Richards Blvd	Signal	55	Е	71	Е
11	N. 7th St / Bannon Street	Signal	21	C	21	C
12	N. 7 <sup>th</sup> St / B Street	Signal	98	F	67	Е
14	N. 7 <sup>th</sup> St / Railyards Blvd	Signal	20	C	320	F
15	N. 6 <sup>th</sup> St / Richards Blvd	Signal	40	D	29	C
16	N. 7 <sup>th</sup> St / C Street	Signal	21	С	26	С

	CUMULATIVE PLUS		BLE 3 I PEAK HOUR	LEVELS (	OF SERVICE	
			Baseline Plu	ıs Project C		
#	Intersection	A.	M Peak Hour		PM Peak	Hour
		Control	Ave Delay (sec/veh)	LOS	Ave Delay (sec/veh)	LOS
6	N. 5th St / Richards Blvd	Signal	19	В	36	D
7	N. 7th St / Richards Blvd	Signal	54	D	148	F
11	N. 7 <sup>th</sup> St / Bannon Street	Signal	17	В	24	C
12	N. 7 <sup>th</sup> St / B Street	Signal	83	F	119	F
14	N. 7 <sup>th</sup> St / Railyards Blvd	Signal	92	F	261	F
15	N. 6 <sup>th</sup> St / Richards Blvd	Signal	21	С	43	D
16	N. 7 <sup>th</sup> St / C Street	Signal	19	С	30	С
НІС	GHLIGHTED locations are RBOC a	access intersection	ns			



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	BASELINE PLU	S RBOC A		SLE 4 K HOUR 95	S <sup>TH</sup> PERCI	ENTILE Q	UEUES	
			AM Pea	k Hour		P	M Peak Ho	ur
#	Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	gth (feet)
		(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95th %
	N. 5th St / Richards Blvd							
	Westbound left	100	10	<25	35	8	<25	45
6	WB thru+rt	325	783	60	130	1,783	200	435
	• EB thru+rt	625	1,540	90	225	860	50	145
	N. 7th St / Richards Blvd		1,5	3 0		000		1.0
	Eastbound left	1801	192	180	275	10	30	130
	EB thru+rt	535	943	360	525	872	445	510
	Northbound left	$\frac{350^2}{150^2}$	41	<25	45	352	110	190
_	NB thru+rt	475	180	140	240	556	225	445
7	Southbound left	140	13	<25	40	87	110	205
	SB thru	250	37	<25	55	119	190	345
	SB right	110	20	<25	55	77	125	200
	Westbound left	190³	488	350	650	297	335	555
	WB thru+rt	1,090	801	205	525	1,040	360	540
	N. 7th St / Bannon Street							
	Southbound left	None	0	0	0		0	0
	SB thru+rt	475	627	210	460	503	230	445
11	Northbound left	120 <sup>4</sup>	161	90	170	31	<25	75
	NB thru	320	103	30	85	738	95	255
	<ul> <li>Eastbound left</li> </ul>	205 <sup>5</sup>	117	85	134	159	195	340
	• Eastbound right	205	128	175	320	124	110	260
	N. 7th St / B Street							
	Southbound left	1406	24	<25	65	91	40	125
	SB thru+rt	190	629	235	290	647	230	290
12	NB left+thru+rt	995	364	780	1,090	1,004	1,005	1,030
	• EB left+thru+rt	1,250	178	110	200	150	120	210
	• WB left+thru	1,100	336	635	940	265	410	710
	WB right turn	1,100	49	145	585	47	25	50
WBI	Richards Blvd Queue		EB Richard	ds Blvd Que	ie		SB N. 7 <sup>th</sup> S	St Queue



<sup>&</sup>lt;sup>1</sup> lane is followed by 185 foot Two-Way left-Turn (TWLT) lane <sup>2</sup> lane continues as (TWLT) to Bannon Street, a total distance of 470 feet <sup>3</sup> lane continues as a TWLT lane to N. 10<sup>th</sup> Street, a total distance of 880 feet <sup>4</sup> lane is followed by a 215 foot TWLT lane to C Street. Total is 335 feet. <sup>5</sup> distance to pedestrian crossing

<sup>&</sup>lt;sup>6</sup> lane is followed by 40 foot bay taper and 60 feet of northbound LT to C Street

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	BASELINE PLU	S RBOC A		(continued) K HOUR 95	S <sup>TH</sup> PERCI	ENTILE Q	UEUES	
			AM Pea	k Hour		P	M Peak Ho	ur
#	Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	ngth (feet)
		(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %
	N. 7th S / Railyards Blvd							
	• NB thru+lt	1,375	124	65	135	403	1,100	1,460
14	• SB thru+rt	1,000	1,020	195	365	883	550	850
	<ul> <li>Eastbound left</li> </ul>	125	256	115	210	659	2,530	3,110
	<ul> <li>Eastbound right</li> </ul>	320	107	55	100	62	40	160
	N. 6 <sup>th</sup> St / Richards Blvd							
	Westbound left	$170^{7}$	103	70	130	34	40	135
15	Westbound thru	535	675	105	250	1,455	290	465
13	• EB thru+rt	325	1,553	430	650	863	225	465
	<ul> <li>Northbound left</li> </ul>	$440^{8}$	78	35	70	336	110	140
	<ul> <li>NB right</li> </ul>	440	35	<25	40	173	140	305
	N. 7th St / C Street							
	Northbound left	609	98	65	120	42	<25	40
16	NB thru	265	227	65	155	535	50	125
10	• SB thru+rt	190	700	250	400	607	310	405
	Eastbound left	$250^{10}$	37	45	125	234	175	325
	Eastbound right	250	184	200	405	163	195	335
WBI	Richards Blvd Queue		EB Richard	ds Blvd Que	ie		SB N. 7 <sup>th</sup>	St Queue



 <sup>&</sup>lt;sup>7</sup> Iane is followed by 185 foot TWLT lane
 <sup>8</sup> distance to Bannon Street
 <sup>9</sup> 60 foot lane followed by 40 foot bay taper and 140 feet SB LT to C Street
 <sup>10</sup> distance to garage

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	CUMULATIVE PI	LUS RBOC		BLE 5 EAK HOUR	95 <sup>TH</sup> PER	CENTILE	QUEUES	
			AM Pea	ık Hour		P	M Peak Ho	ur
#	Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	gth (feet)
		(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %
	N. 5 <sup>th</sup> St / Richards Blvd							
_	Westbound left	100	240	110	145	265	115	155
6	WB thru+rt	325	1,205	150	320	1,315	185	365
	EB thru+rt	625	1,450	55	130	1,070	315	590
	N. 7th St / Richards Blvd		1,100	1 00	100	1,070	0.10	
	Eastbound left	$180^{1}$	175	165	265	5	<25	60
	EB thru+rt	535	1,190	360	540	1,075	455	520
	Northbound left	$\frac{150^2}{}$	225	90	155	230	235	515
_	NB thru+rt	475	380	200	335	380	395	575
7	Southbound left	140	35	40	135	80	80	215
	SB thru	250	210	185	310	400	2,025	2,995
	SB right	110	30	45	135	50	40	135
	Westbound left	$190^{3}$	395	235	395	495	715	955
	• WB thru+rt	1,090	1,180	245	450	1,125	440	1,065
	N. 7th St / Bannon Street							
	Southbound left	150	30	30	85	100	70	155
	SB thru+rt	475	750	145	260	1,145	150	295
11	Northbound left	120 <sup>4</sup>	70	45	95	50	50	120
	NB thru	320	410	45	105	690	150	290
	Eastbound left	2055	115	150	250	100	155	270
	EB thru+right	205	145	45	115	145	150	295
	N. 7th St / B Street							
	Southbound left	$140^{6}$	165	115	150	265	120	130
	SB thru+rt	190	600	200	290	1,005	230	305
	Northbound left	unk	55	45	120	85	35	80
12	NB thru+rt	995	550	270	425	1,145	490	505
	Eastbound left	unk	95	70	145	130	100	155
	EB thru+rt	unk	430	730	1,215	480	1,935	2,740
	• WB left	1,100	720	465	725	405	460	820
	• WB thru+rt	1,100	710	380	690	650	630	1,040
WB	Richards Blvd Queue		EB Richar	ds Blvd Que	ue		SB N. 7 <sup>th</sup>	St Queue



<sup>&</sup>lt;sup>1</sup> lane is followed by 185 foot TWLT lane <sup>2</sup> lane continues as TWLT to Bannon Street, a total distance of 470 feet <sup>3</sup> land continues as a TWLT lane to N. 10<sup>th</sup> Street, a total distance of 880 feet <sup>4</sup> lane is followed by 215 foot TWLT lane to C Street. Total is 335 feet.

<sup>&</sup>lt;sup>5</sup> distance to pedestrian crossing

<sup>&</sup>lt;sup>6</sup> lane is followed by 40 foot bay taper and 60 feet of northbound LT to C Street

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	CUMULATIVE PI	LUS RBOC		(continued) CAK HOUR	95 <sup>TH</sup> PER	CENTILE	QUEUES	
			AM Pea	k Hour		P	M Peak Ho	ur
#	Intersection	Storage	Volume	Queue len	gth (feet)		Queue ler	ngth (feet)
77	Imersection	(feet)	(vph)	Average	95 <sup>th</sup> %	Volume (vph)	Average	95 <sup>th</sup> %
	N. 7th S / Railyards Blvd							
	<ul> <li>Northbound left</li> </ul>	unk	315	220	255	270	150	260
	• NB thru+lt	1,375	875	545	995	1,040	3,800	5,190
	<ul> <li>Southbound left</li> </ul>	unk	25	<25	60	40	60	140
1.4	• SB thru+rt	1,000	940	250	375	910	205	350
14	<ul> <li>Eastbound left</li> </ul>	125	295	175	240	375	185	210
	• EB thru	320	435	1,160	1,815	500	4,440	5,940
	EB right	unk	425	265	830	565	4,155	6,010
	Westbound left	unk	455	275	275	385	275	285
	• WB thru+rt	unk	425	1,240	1,315	390	1,455	1,905
	N. 6th St / Richards Blvd							
	Westbound left	$170^{7}$	150	115	225	185	105	200
1.5	Westbound thru	535	1,260	135	335	1,205	125	225
15	EB thru+rt	325	1,795	205	445	1,320	520	595
	Northbound left	4408	190	95	140	420	270	440
	NB right	440	30	35	105	20	<25	65
	N. 7th St / C Street							
	Northbound left	609	110	80	130	50	40	100
1.0	NB thru	265	445	45	155	465	80	180
16	SB thru+rt	190	925	135	315	1,165	250	400
	Eastbound left	25010	35	45	120	275	155	270
	Eastbound right	250	40	55	225	120	150	295
WB	Richards Blvd Queue		EB Richard	ds Blvd Que	ie		SB N. 7 <sup>th</sup> S	St Queue

 $<sup>^7</sup>$  lane is followed by 185 foot TWLT lane  $^8$  distance to Bannon Street



<sup>&</sup>lt;sup>9</sup> 60 foot lane followed by 40 foot bay taper and 140 foot southbound LT at B Street <sup>10</sup> distance to garage

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### Access Assessment

The following key findings were made based on review of this information and observation of simulation results.

Levels of Service at Access Intersections. The three access intersections are projected to operate with Levels of Service that satisfy the City of Sacramento's minimum LOS E standard under both Baseline plus RBOC and Cumulative plus RBOC conditions. However as noted earlier, while the combinations of traffic volume and intersection capacity at each location project LOS E or better conditions, the projected average delays to not fully account for the effects of queueing from downstream intersections that may delay traffic into and out of the site. Average delays could be longer.

Queues on Richards Blvd between N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street. The extent to which anticipated left turn queues can be accommodated within the space available between the N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street intersections. The RBOC project striping plans in each area propose designated turn lanes and Two-Way Left-Turn (TWLT) lane. Table 6 compares these distances with the anticipated queue lengths under Baseline plus Project and Cumulative plus Project conditions.

As indicated, the 95<sup>th</sup> percentile queues can be accommodated in the area between the two intersections, although the queues will extend into the TWLT lane area. Observation of the simulation results reveals that the turn lane queues do not spillover and interfere with through traffic. No alternatives for these intersection have been assessed.

	R	ICHARD	BLVD	QUEUE	TABL S – N. (	E 6 5 <sup>th</sup> STREET T	O N. 7 <sup>th</sup> S7	TREET			
Westb	ound Left T	Turn Lan	e at N. 6	th Street	t	Eastbou	ınd Left Tı	ırn Lane	at N.	7 <sup>th</sup> Stre	et
		95 <sup>th</sup> Pe	rcentile	Queue (	(feet)			95 <sup>th</sup>	Percer	ıtile Qu	eue
Element	Length	Baselin	e plus	Cumu	lative	Element	Length	Base	line	Cumı	ılative
Liement	(feet)	RBC	OC	plus R	BOC	Liement	(feet)	plus R	BOC	plus I	RBOC
		AM	PM	AM	PM			AM	PM	AM	PM
Turn Lane	170					Turn Lane	180				
TWLT lane	185					TWLT lane	35				
Total	355	130	135	225	200	Total	215	275	130	265	60

Queues on N. 7<sup>th</sup> Street between Richards Blvd and Bannon Street. The extent to which anticipated left turn queues can be accommodated within the space available between these intersections was evaluated.

The project striping plans in each area proposed designated turn lanes and TWLT lane. Table 7 compares these distances with the anticipated queue lengths under Baseline plus Project and Cumulative plus Project conditions. As indicated, in the near term the entire 470 feet of separation between the intersections would all be available for the northbound queue. The projected queue is far less than this distance When Bannon Street is extended easterly in the future, the area of N. 7<sup>th</sup> Street between Richards Blvd and Bannon Street might be striped with separate left turn lanes at each intersection, and another 170 feet of TWLT separation could be available between turn lanes. Under the Cumulative plus Project conditions the northbound 95<sup>th</sup>



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percentile queue may reach Bannon Street in the p.m. peak hour, and the sum of northbound and southbound queues could exceed the available distance between intersections. However, this conflict did not appear in viewing the simulation.

TABLE 7 N. 7 <sup>th</sup> STREET QUEUES – RICHARDS BLVD TO BANNON STREET											
Northbound Left Turn Lanes at Richards Blvd Southbound Left Turn Lane at Bannon Street											
	95 <sup>th</sup> ]	Percenti	le Queue	(feet)			95th Percentile Queue				
Element	Length (feet)	Baseline plus RBOC		Cumulative plus RBOC		Element	Length (feet)	Baseline		Cumulative	
								plus RBOC		plus RBOC	
		AM	PM	AM	PM			AM	PM	AM	PM
Turn Lane	150					Turn Lane	150				
TWLT lane	$170^{1}$					Separation	170				
Total	320	45	190	155	515	Total	320	n.a.	n.a.	100	155
Total Both Queues 45 190 255 670											
<sup>1</sup> separation to end of future SB left turn lane at Bannon Street											

Queues on N. 7<sup>th</sup> Street between Bannon Street and C Street. The extent to which anticipated left turn queues can be accommodated within the space available between these intersections was evaluated.

The project striping plans in each area proposes a designated turn lanes and TWLT lane to the limit of the C Street intersection. Table 8 compares these distances with the anticipated queue lengths under Baseline plus Project and Cumulative plus Project conditions. As shown the projected queues approaching Bannon Street do not extend to C Street.

Westbound Bannon Street. In addition to the effects of intersection traffic operations, the queue in the northbound left turn lane is related to the operation of westbound Bannon Street. In this area 135 feet of curb-side drop-off will be provided on the north side of the street between N. 7<sup>th</sup> Street and the pedestrian crossing. This area can accommodate 5-6 vehicles concurrently. The raised pedestrian crossing is located 200 feet beyond N. 7<sup>th</sup> Street. Bannon Street has been narrowed to two travel lanes (i.e., 24 feet) with curb bulb outs. More than 1,000 pedestrian crossing per hour are anticipated. Formal automobile traffic or pedestrian traffic controls are not planned. Automobiles would have the right of way at the crossing but cannot legally enter the crossing when occupied by pedestrians. Under these circumstances it is likely that a queue of westbound traffic will extend back from the crossing towards N. 7<sup>th</sup> Street, and the simulation confirms queuing. It is possible that northbound traffic turning left may sit through more than one cycle and that the projected queue could be longer than estimated.



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TABLE 8 N. 7 <sup>th</sup> STREET QUEUES – BANNON STREET TO C STREET											
Northbound Left Turn Lanes at Bannon Street						Southbound Left Turn Lane at C Street					
	95th Percentile Queue (feet)						95 <sup>th</sup> Percentile Queue				
Element	Length (feet)	Baseline plus RBOC		Cumulative plus RBOC		Element	Length (feet)	Baseline		Cumulative	
								plus RBOC		plus RBOC	
		AM	PM	AM	PM			AM	PM	AM	PM
Turn Lane	120					Turn Lane	n.a.				
TWLT lane	215					Separation	n.a.				
Total	335	170	75	95	120	Total	n.a.	n.a.	n.a.	n.a.	n.a.

Queues on N. 7<sup>th</sup> Street between C Street and B Street. The extent to which anticipated left turn queues can be accommodated within the space available between these intersections was evaluated, as noted in Table 9.

The segment of N. 7<sup>th</sup> Street in this area includes a 140 foot-long southbound left turn lane at B Street, and from that point northerly the street is striped with a reversing bay taper and 60 foot northbound left turn lane at C Street. The distance from the limit of the C Street intersection to the left turn lane limit line at B Street is 240 feet. Table 9 compares this distance with the anticipated queue lengths under Baseline plus Project and Cumulative plus Project conditions.

At this location the two abutting queues might need to be accommodated concurrently in this 240-foot area. In the a.m. peak hour the sum of concurrent queues is projected to be more than 240 feet under Cumulative plus Project conditions. Because the peak queues are not forecast to occur concurrently, the simulation runs revealed that no spillover occurred under Existing plus Project condition but that southbound through traffic was impeded by spillover queues under Cumulative plus Project conditions.

TABLE 9 N. 7 <sup>th</sup> STREET QUEUES – C STREET TO B STREET											
Northbound Left Turn Lanes at C Street Southbound Left Turn Lane at B Street											
95th Percentile Queue (feet)					95th Percentile Queue						
Element	Length (feet)	Baseline plus RBOC		Cumulative plus RBOC		Element	Length (feet)	Baseline plus RBOC		Cumulative plus RBOC	
		AM	PM	AM	PM			AM	PM	AM	PM
Turn Lane	60					Turn Lane	140				
Bay Taper	40					Bay Taper	40				
Turn Lane	140					Turn Lane	60				
Total	240	120	40	130	100	Total	240	65	125	150	130
Total Both Queues		185	165	280	230						



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*Alternatives.* In the short term conditions at this location would change with the implementation of improvements that are not included in the scope of the RBOC project but will be installed by other Railyard area projects (i.e., widening of N. 7<sup>th</sup> Street south of B Street).

Alternatives to full signalized access at the N. 7<sup>th</sup> Street / C Street intersection were considered but rejected. These alternatives are:

- Unsignalized N. 7<sup>th</sup> Street / C Street intersection with full access
- Partial traffic signal with northbound left turn prohibited

Decisions regarding access to this location needs to consider the opportunities for alternative access that may be absent today but will be available in the future, as well as the effects of alternative traffic patterns through the RBOC site. In the short term eliminating northbound left turn at C street would add traffic onto Bannon Street and which could exacerbate conflicts with the drop-off and loading functions on that street and at the pedestrian crossing. In the long term the anticipated extensions of 5<sup>th</sup> and 6<sup>th</sup> Streets to Railyards Blvd could compensate for the absence of a northbound left turn at C Street if necessary, but in the near term extending N. 6<sup>th</sup> Street to B Street is beyond the scope of the RBOC project. The removal of a traffic signal at C Street would result in a large volume of traffic crossing the LRT tracks at an uncontrolled crossing. This alternative is unlikely to be accepted by SacRT or by the California Public Utilities Commission.

Westbound C Street. As with the N. 7<sup>th</sup> Street / Bannon Street traffic signal, the flow of westbound traffic on C Street would be a consideration in the evaluation of the C Street signal. In this case the gated access to the parking lots and to the parking structure would be the primary issue. The extent to which traffic from these locations could queue back to N. 7<sup>th</sup> Street is related to the gate capacity and resulting vehicle headways. The two garage gates employ proximity sensors to detect and validate the license plate of an entering vehicle. Under this system each gate has a practical capacity of about 600 vehicles per hour (vph), and the combined capacity of the garage's four gates is 2,400 vehicles per hour. At this rate the site's 1,630 spaces could be filled in 40 minutes. A practical matter the flow rate from N. 7<sup>th</sup> Street and from Richards Blvd to the garages is somewhat metered by signal timing at the adjoining intersections, and the garage entrance capacity at each entrance (i.e., 1,200 vph) exceeds the flow rate out of the traffic signals into RBOC. As a result, queuing from the garages back to N. 7<sup>th</sup> Street is not anticipated in the simulation.

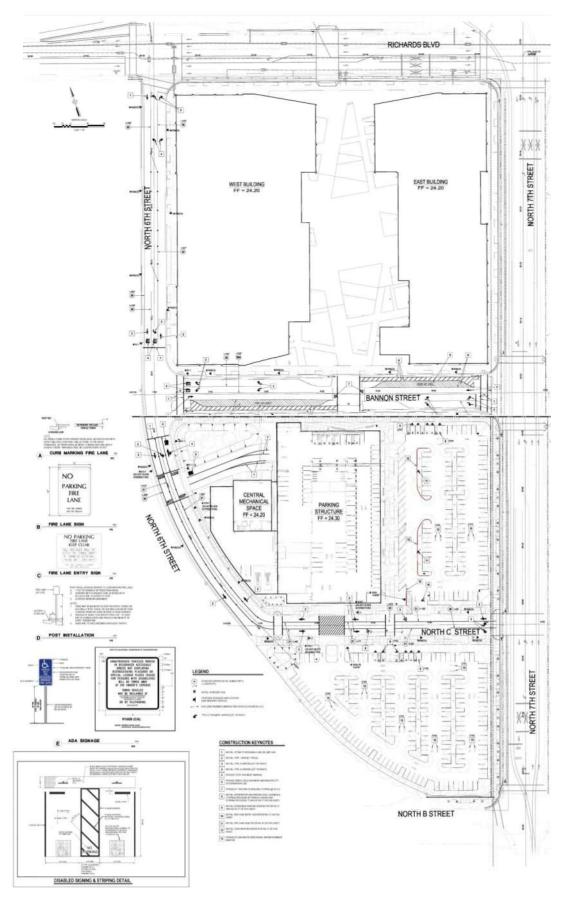
*Eastbound C Street.* In the evening traffic will flow out of the garage and parking lots and into the N. 7<sup>th</sup> Street intersection. While delays to eastbound RBOC traffic do not affect the flow of background traffic on N. 7<sup>th</sup> Street, the queue of southbound through traffic on N. 7<sup>th</sup> Street does affect outbound RBOC traffic. As noted in Tables 4 and 5 those queues are projected to result in eastbound queues on C Street that extend back to the garage entrance.

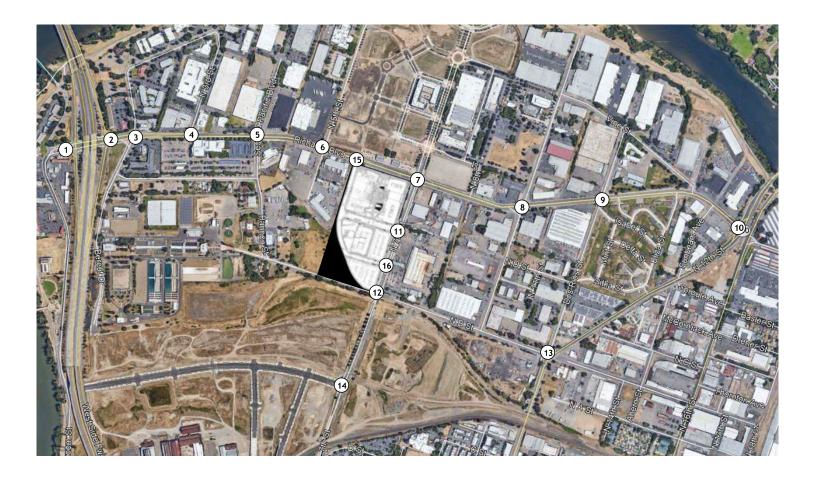




KD Anderson & Associates, Inc. Transportation Engineers
4705-025 RA 7/8/2020 VICINITY MAP

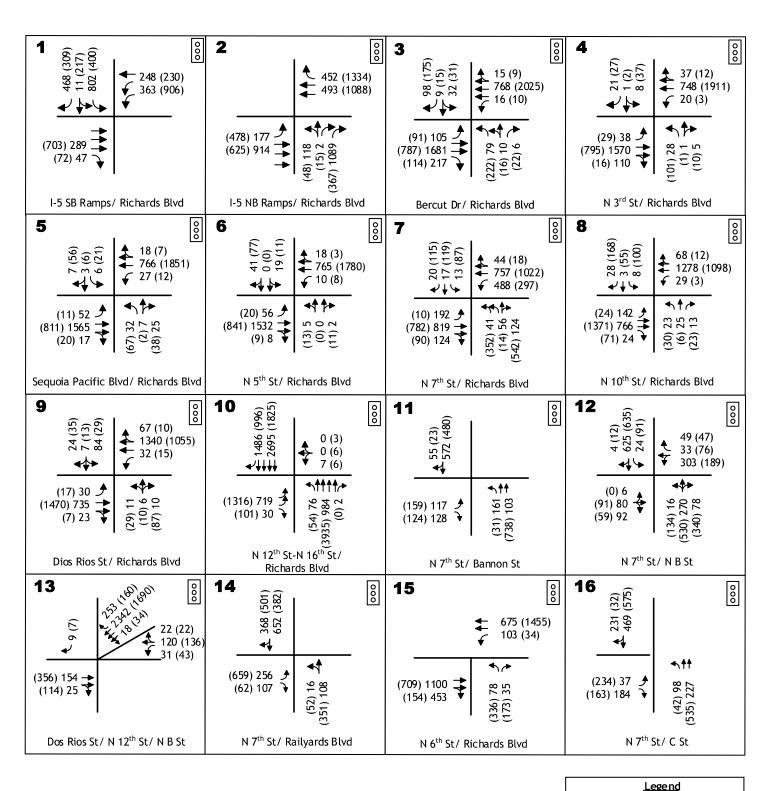
ros-o25 RA 7/8/2020 figure 1

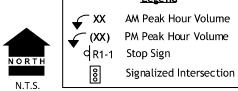


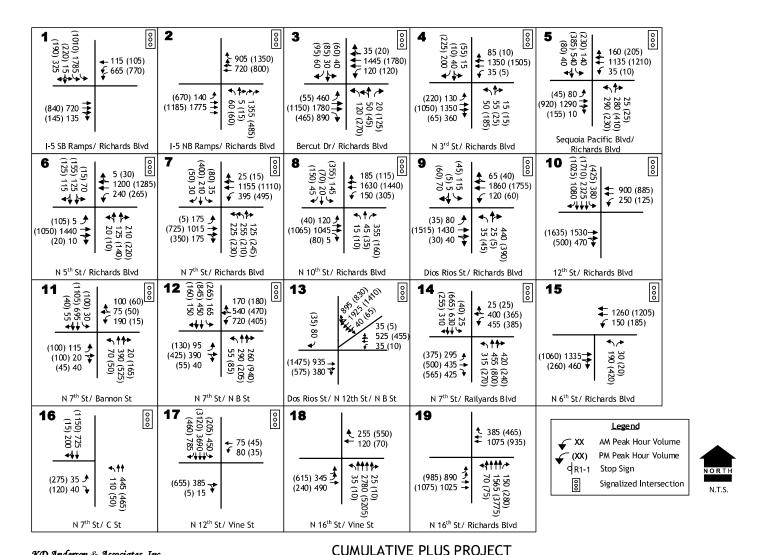


KD Anderson & Associates, Inc. Transportation Engineers
4705-025 RA 7/8/2020 INTERSECTION STUDY LOCATIONS

figure 3







KD Anderson & Associates, Inc.

Transportation Engineers

4705-025 RA 7/8/2020 figure 5

# Attachment F

Signal Design Concept Report

# SIGNAL DESIGN CONCEPT REPORT

### RICHARDS BLVD OFFICE COMPLEX

Sacramento, CA



Prepared By:

KD Anderson & Associates, Inc.

3853 Taylor Road, Suite G Loomis, CA 95650 (916) 660-1555

	ACCEPTED BY:	
		Date
)	DAVID EDROSOLAN, P.E., T.E. CITY TRAFFIC ENGINEER CITY OF SACRAMENTO	

DRAFT November 4, 2020

4705-025

# RICHARDS BLVD OFFICE COMPLEX

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## **INTRODUCTION**

This report provides analysis of proposed new signalized intersections and existing signalized intersections adjacent to the Richards Blvd Office Complex (RBOC) project in the City of Sacramento. The project site is located south of Richards Blvd and west of N. 7<sup>th</sup> Street. The RBOC proposes signalized vehicle access at three new intersections and will modify two existing signalized intersections in conjunction with proposed frontage improvements on Richards Blvd and on N. 7<sup>th</sup> Street. The intersection locations are summarized below. Figure 1 displays the subject intersections and the RBOC site.

- N. 7<sup>th</sup> Street / Bannon Street (new)
- N. 7<sup>th</sup> Street / C Street (new)
- N. 6<sup>th</sup> Street / Richards Blvd (new)
- Richards Blvd / N. 7<sup>th</sup> Street (modification)
- N. 7<sup>th</sup> Street / N. B Street (modification)

This report is intended to summarize signal design concept elements for improvements to each intersection for review and approval by City staff prior to preparation of construction documents for the traffic signal installations and/or modifications. The report presents traffic volume projections for the intersections, associated operating levels of service, vehicle queue estimates and discussion of pedestrian and bicycle facilities. Also appended to this report is a memorandum prepared by Kimley-Horn which outlines in detail the light rail preemption operations for the two new traffic signals proposed for N. 7<sup>th</sup> Street.

This report includes and makes use of a traffic operations analysis technical memo (*Traffic Operational Analysis for RBOC DCR's, KD Anderson & Associates*) prepared for and in advance of this Design Concept Report. The analysis provides SimTraffic simulation to evaluate the operation of existing and proposed traffic signals in the area of the RBOC as required by the City of Sacramento to support the Design Concept Report. The operational analysis has been previously reviewed and commented on by City staff. The analysis utilized traffic volume forecasts created for the RBOC EIR and subsequently supplemented for the Department of General Services (DGS) were adjusted to account for the quantity of employment and on-site parking now proposed, as well as the location of site access, parking and drop-off /loading. SimTraffic networks created for the RBOC EIR and subsequent document were then adjusted to account for the current access and internal circulation proposal. Resulting traffic operations expressed in terms of intersection delays and peak vehicle queues were projected and used to evaluate the feasibility of proposed access as well as internal circulation.

RBOC on-site employment is expected to ultimately reach 5,000 persons. The RBOC includes a 1,294 space parking garage with access to C Street between N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street and to N. 6<sup>th</sup> Street between Richards Blvd and Bannon Street, as well as surface parking for visitors and employees with access to C Street. The RBOC includes designated drop-off and loading areas on Bannon Street as well as a pedestrian crossing linking RBOC's office towers with the parking structure. No direct access to the project's parking areas is planned via Bannon Street.



#### **SETTING**

#### Roads

**Richards Blvd** is a 4-lane arterial street which extends from I-5 in the west to SR 160 in the east, a distance of approximately 1.5 miles. The majority of the roadway provides a continuous center turn lane. The posted speed limit ranges from 35 - 40 mph.

The project will construct a new N. 6<sup>th</sup> Street intersection with Richards Blvd located approximately 600' west of N. 7<sup>th</sup> Street and 400' east of N. 5<sup>th</sup> Street. The south side of Richards Blvd will be widened and reconstructed between the new N. 6<sup>th</sup> Street intersection and N. 7<sup>th</sup> Street. Existing bike lanes will also be enhanced between these intersections and will include 6' wide lanes with a 3' wide bike lane buffer on both the north and south sides of the street.

Richards Blvd will be restriped between the N. 6<sup>th</sup> and N. 7<sup>th</sup> Street intersections to eliminate the existing 3<sup>rd</sup> westbound lane. This lane currently extends west from N. 7<sup>th</sup> Street and is dropped at the location of the new N. 6<sup>th</sup> Street alignment. The existing bus pull out will remain in place on the north side of the street. These striping modifications will eliminate the lane drop merge maneuvers, provide width for the bike lane buffer and facilitate traffic signal improvements at the N. 6<sup>th</sup> Street intersection. The balance of the existing pavement area and pull out will be available for RT busses and passenger drop-off to the light rail station.

A bus stop will be located on the south side of Richards Blvd east of N. 6<sup>th</sup> Street. This will include a bus pad for loading and unloading of passengers.

**N.** 7<sup>th</sup> Street is designated as a collector street south of Richards Blvd to N. B Street and an arterial street south of N. B Street. N. 7<sup>th</sup> Street extends south from Richards Blvd through the downtown for a distance of approximately 2 miles and terminates at T Street. The posted speed limit is 35 mph. The roadway currently provides two northbound and one southbound travel lane and a continuous center turn lane between Richards Blvd and N. B Street. The 2<sup>nd</sup> southbound lane is currently striped out to prohibit vehicles and the southbound light rail track alignment also follows this lane. South of N. B Street the roadway is a 2-lane street.

The Sacramento Regional Transit District (RT) operates the Green Line light rail line along N. 7<sup>th</sup> Street. The Green Line is in-street running from N. B Street to Richards Blvd and terminates at the Township 9 LRT station located at the northwest quadrant of the Richards Blvd / N. 7<sup>th</sup> Street intersection. The northbound and southbound tracks transition to a single track within the N. 7<sup>th</sup> Street / N. B Street intersection and continue south on the east side of N. 7<sup>th</sup> Street adjacent to the northbound vehicle lane.

The RBOC project will construct new Bannon Street and N. C Street intersections with N. 7<sup>th</sup> Street to serve the project. The Bannon Street intersection will be located 550' south of Richards



Blvd. The N. C Street intersection will be located 400' south of Bannon Street and 300' north of N. B Street. The west side of N. 7<sup>th</sup> Street will be widened and reconstructed between N. B Street and Richards Blvd. This will include a 2-way cycle track facility to replace the existing southbound Class II bike lane. A 2' wide raised median will separate the cycle track from the southbound vehicle lanes and RT track. The cycle track will provide 5' lanes separated by a 2' wide striped median. Cycle track operations will be controlled by the traffic signals at the Bannon Street and N. C Street intersections.

### **Alternative Transportation Modes**

**Transit Service.** RT bus and light rail service is provided on Richards Blvd and via the Township 9 LRT station immediately adjacent to the site. A new bus stop will be located on the south side of Richards Blvd immediately east of N. 6<sup>th</sup> Street.

**Pedestrian Facilities.** Continuous sidewalks are provided on Richards Blvd and on N. 7<sup>th</sup> Street in the vicinity of the project site. The RBOC project will widen existing sidewalk facilities adjacent to the site. A 24' sidewalk will be provided on the Richards Blvd frontage and a 16' sidewalk along the N. 7<sup>th</sup> Street frontage. Expanded pedestrian crossings 15' in width will be provided at the N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street intersections for pedestrian circulation to and from the RBOC site and the Township 9 LRT station.

**Bicycle Facilities**. Class II on-street bike lanes are currently provided on both Richards Blvd and N. 7<sup>th</sup> Street adjacent to the RBOC site. As previously discussed, the project will construct enhanced bicycle facilities on both streets adjacent to the site. Six foot Class II bike lanes with a 3' wide striped buffer will be provided on Richards Blvd. A 2-way cycle track with 5' lanes and a raised 2' wide island buffer will be provided on the west side of N. 7<sup>th</sup> Street.

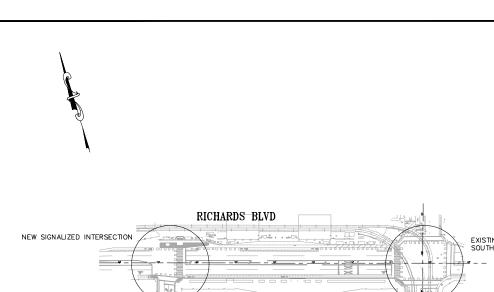
### **Adjacent Land Uses**

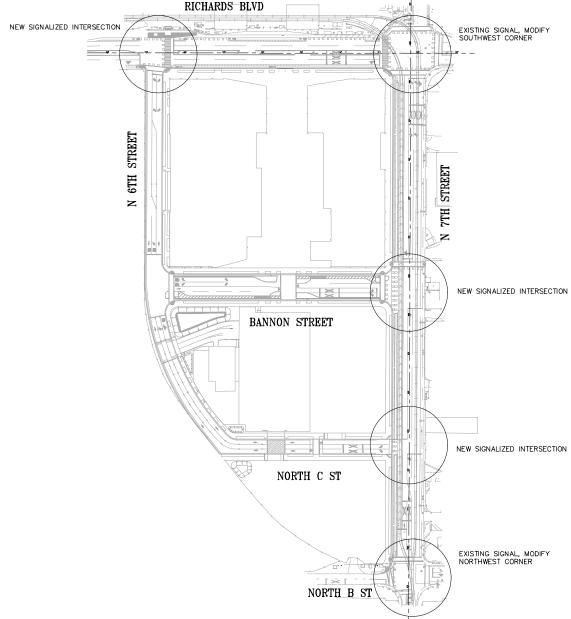
Existing development adjacent to the RBOC project and subject intersections consists of the Township 9 LRT station to the north, industrial uses on the east side of N.7<sup>th</sup> Street and industrial uses to the west of the proposed N. 6<sup>th</sup> Street alignment. Property on N. B Street adjacent to the south side of the RBOC site is undeveloped.

### **Traffic Volumes**

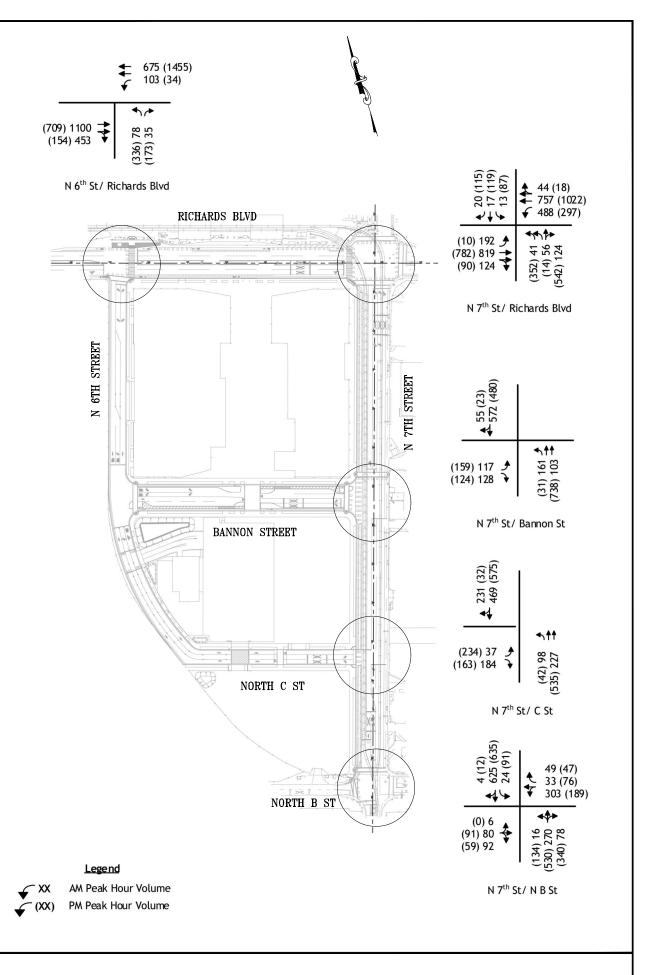
Figures 2 and 3 display baseline plus project and cumulative with project peak hour intersection volumes, respectively. Please refer to the appended operational analysis for additional detail on traffic volume projections. Pedestrian volumes have also been estimated for the primary circulation routes. These consist of Richards Blvd crossings at the N. 6<sup>th</sup> Street and N. 7<sup>th</sup> Street intersections to and from the RBOC site and the Township 9 LRT station. Approximately 600 peak hour pedestrians are projected at the N. 6<sup>th</sup> Street intersection and 900 at the N. 7<sup>th</sup> Street intersection.

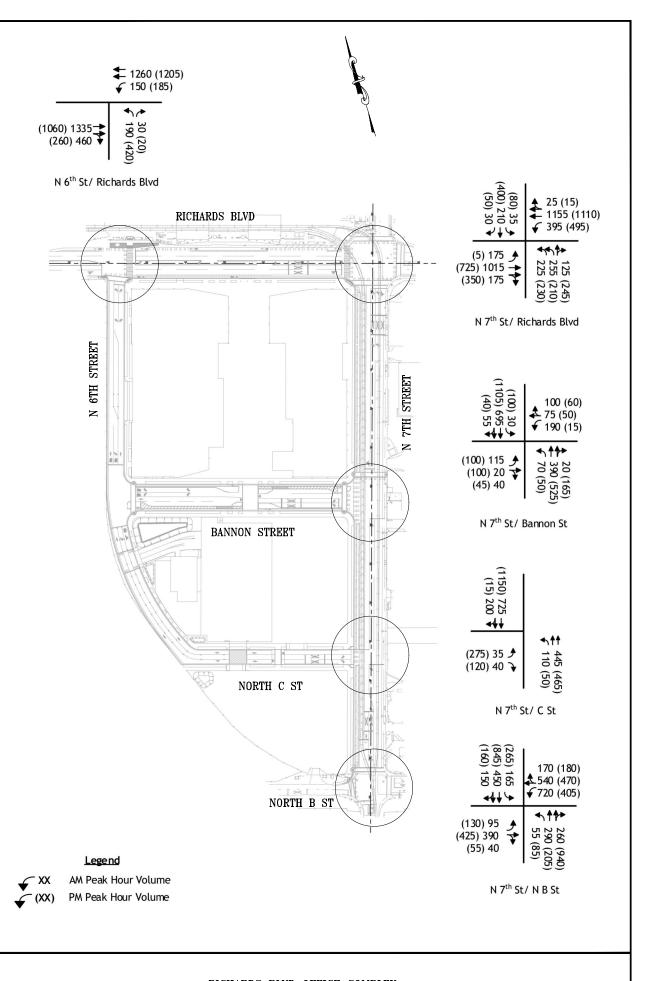












# RICHARDS BLVD / N. 7<sup>TH</sup> STREET INTERSECTION

#### PROJECTED INTERSECTION OPERATIONS

### **Intersection Levels of Service**

Projected intersection operations are summarized in Tables 1 and 2 for baseline plus project and cumulative with project conditions, respectively. As shown in Tables 1, level of service (LOS) E is projected in both the a.m. and p.m. peak hours under baseline plus project conditions. This is within acceptable operating standards identified for the intersection. Under long term cumulative conditions, p.m. peak hour intersection operations are projected at LOS F, exceeding identified standards.

TABLE 1 BASELINE PLUS RBOC LEVELS OF SERVICE								
	Acceptable	AM Pea	ak Hour	PM Peak Hour				
Intersection	LOS	LOS	Delay	LOS	Delay			
N. 7 <sup>th</sup> Street / Richards Blvd	A-E	Е	55	Е	71			

TABLE 2 CUMULATIVE PLUS RBOC LEVELS OF SERVICE								
	Acceptable	AM Pea	ık Hour	PM Peak Hour				
Intersection	LOS	LOS	Delay	LOS	Delay			
N. 7 <sup>th</sup> Street / Richards Blvd	A - E	D	54	F	148			

#### **Vehicle Queues**

Tables 3 and 4 summarize projected vehicle queues at the intersection. As shown, projected vehicle queues can be accommodated within the turn pocket lengths with the exception of the existing southbound left and right turn lanes under both background and cumulative plus project conditions.



TABLE 3 BASELINE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES										
	AM Peak Hour			P	M Peak Ho	ur				
Intersection	Storage Volume Queue length (feet)		Volume	Queue ler	gth (feet)					
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %			
N. 7th St / Richards Blvd										
Eastbound left	$170^{1}$	192	180	275	10	30	130			
Northbound left	620 <sup>2</sup>	41	<25	45	352	110	190			
Southbound left	140	13	<25	40	87	110	205			
SB right	110	20	<25	55	77	125	200			
Westbound left	1903	488	350	650	297	335	555			

TABLE 4 CUMULATIVE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES										
		AM Pea	k Hour		P	M Peak Ho	ur			
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	ngth (feet)			
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %			
N. 7th St / Richards Blvd										
Eastbound left	$170^{1}$	175	165	265	5	<25	60			
Northbound left	$620^{2}$	225	90	155	230	235	515			
Southbound left	140	35	40	135	80	80	215			
SB right	110	30	45	135	50	40	135			
Westbound left	190³	395	235	395	495	715	955			

## SIGNAL DESIGN SUMMARY

Exhibits TS-1 and TS-2 display the traffic signal modifications proposed for the Richards Blvd / N. 7<sup>th</sup> Street intersection. The attached Striping Plan exhibit displays intersection geometrics, bicycle facilities, pedestrian crosswalk striping and lane dimensions. The majority of the existing signal equipment at the intersection will remain in place. No change to existing signal phasing is proposed. Equipment at the southwest corner will be replaced to accommodate project frontage improvements. It is proposed to reuse the existing video detection camera and emergency vehicle detector at the southwest corner. Eastbound advance detector loops will be replaced and located for a 35 mph design speed.



## **Truck Turn Paths**

Design vehicle turning movement paths are displayed on the attached exhibit. An eastbound right turn movement from Richards Blvd to N. 7<sup>th</sup> Street is displayed around the proposed southwest corner which will be reconstructed. As shown, a WB-50 design vehicle can satisfactorily negotiate this turn.

## **Pedestrian Crossing Locations**

Crosswalks and pedestrian equipment currently provided across all legs of the intersection. The west side crosswalk will be expanded to 15' in width and striped with continental solid bars for higher visibility.

### **Bicycle Movements**

Colored bicycle markings are proposed across all legs of the intersection on the inside of the pedestrian crosswalk markings to direct bicycle turning traffic to utilize the pedestrian signals. The northbound cycle track lane will terminate at the south side of the intersection. Colored bicycle markings are also proposed to direct bikes to the southwest corner of the intersection and utilize pedestrian signals to continue across the intersection. Additional signing and pavement markings are also proposed to indicate that the cycle track lane is ending at the intersection. This is displayed on exhibit TS-1.

# N. 7<sup>TH</sup> STREET / N. B STREET INTERSECTION

#### PROJECTED INTERSECTION OPERATIONS

### **Intersection Levels of Service**

Projected intersection operations are summarized in Tables 5 and 6 for baseline plus project and cumulative with project conditions, respectively. As shown in Tables 6, LOS F and E are projected in the a.m. and p.m. peak hours under baseline plus project conditions. Under long term cumulative conditions, LOS F is projected during both the morning and afternoon peak hours.

TABLE 5 BASELINE PLUS RBOC LEVELS OF SERVICE							
	Acceptable	AM Pea	ık Hour	PM Pea	PM Peak Hour		
Intersection	LOS	LOS	Delay	LOS	Delay		
N. 7 <sup>th</sup> Street / N. B Street	A - E	F	98	Е	67		



TABLE 6 CUMULATIVE PLUS RBOC LEVELS OF SERVICE								
	Acceptable	AM Peak Hour PM Peak Hour						
Intersection	LOS	LOS	Delay	LOS	Delay			
N. 7 <sup>th</sup> Street / N. B Street	A - E	F	83	F	119			

## **Vehicle Queues**

Tables 7 and 8 summarize projected vehicle queues at the intersection. As shown, projected left turn queues can generally be accommodated within the turn pocket lengths under both background and cumulative plus project conditions. The southbound left turn pocket on N. 7<sup>th</sup> Street is projected to be slightly exceeded (5') under cumulative p.m. peak hour conditions.

TABLE 7 BASELINE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES									
	AM Peak Hour PM Peak Hour				ur				
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue length (feet)			
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %		
N. 7th St / B Street									
Southbound left	1251	24	<25	65	91	40	125		
WB right turn	1,100	49	145	585	47	25	50		

TABLE 8 CUMULATIVE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES								
		AM Pea	k Hour		P	M Peak Ho	ur	
Intersection	Storage Volume		Queue len	gth (feet)	Volume	Queue length (feet)		
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %	
N. 7th St / B Street								
Southbound left	125 <sup>1</sup>	165	115	150	265	120	130	
• WB left	1,100	720	465	725	405	460	820	

#### **SIGNAL DESIGN SUMMARY**

Exhibit TS-3 displays the traffic signal modifications proposed for the N. 7<sup>th</sup> Street / N. B Street intersection. The attached Striping Plan exhibit displays intersection geometrics, bicycle facilities, pedestrian crosswalk striping and lane dimensions. The majority of the existing signal equipment at the intersection will remain in place. No change to existing signal phasing is proposed. The 1-B pole and PBA post at the northwest corner will be replaced to accommodate



project frontage improvements. The existing mast arm pole and equipment can remain in place on this corner. Two pull boxes will need to be relocated behind the new curb line.

## **Pedestrian Crossing Locations**

Crosswalks and pedestrian equipment is currently provided across all legs of the intersection.

## **Bicycle Movements**

Pedestrians and bicycles currently share the sidewalk on the west side of N. 7<sup>th</sup> Street south of N. B Street. Colored bike markings are proposed for the west side of the intersection to direct northbound bicyclists to the beginning of the cycle track at the northwest corner of the intersection.

## N. 7<sup>TH</sup> STREET / BANNON STREET INTERSECTION

#### PROJECTED INTERSECTION OPERATIONS

## **Intersection Levels of Service**

Projected intersection operations are summarized in Tables 9 and 10 for baseline plus project and cumulative with project conditions, respectively. As shown, satisfactory LOS C or better is projected in both the a.m. and p.m. peak hours under both baseline and cumulative plus project conditions. This is within acceptable operating standards identified for the intersection.

TABLE 9 BASELINE PLUS RBOC LEVELS OF SERVICE								
	Acceptable	AM Peak Hour PM Peak Hour						
Intersection	LOS	LOS	Delay	LOS	Delay			
N. 7 <sup>th</sup> Street / Bannon Street	A - E	С	21	С	21			

TABLE 10 CUMULATIVE PLUS RBOC LEVELS OF SERVICE								
	Acceptable	AM Pea	ak Hour	PM Peak Hour				
Intersection	LOS	LOS	Delay	LOS	Delay			
N. 7th Street / Bannon Street	A - E	В	17	С	24			



## **Vehicle Queues**

Tables 11 and 12 summarize projected vehicle queues at the intersection. As shown, projected northbound left turn vehicle queues can be accommodated within the turn pocket length and center lane area. Eastbound queues on Bannon Street are projected to extend past the mid-block pedestrian crosswalk.

TABLE 11 BASELINE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES									
		AM Pea	k Hour		P	M Peak Ho	ur		
Intersection	Storage	Volume	olume Queue length (feet)		Volume	olume Queue length (fe			
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %		
N. 7th St / Bannon Street									
Northbound left	$120^{1}$	161	90	170	31	<25	75		
Eastbound left	205 <sup>2</sup>	117	85	134	159	195	340		
Eastbound right	205	128	175	320	124	110	260		

TABLE 12 CUMULATIVE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES									
		AM Pea	k Hour		F	M Peak Ho	ur		
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	gth (feet)		
	(feet)	(feet) (vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %		
N. 7th St / Bannon Street									
Northbound left	$120^{1}$	70	45	95	50	50	120		
Eastbound left	205 <sup>2</sup>	115	150	250	100	155	270		
• EB thru+right	205	145	45	115	145	150	295		

#### SIGNAL DESIGN SUMMARY

Exhibit TS-4 displays the proposed traffic signal design for the Bannon Street / N. 7<sup>th</sup> Street intersection. The attached Striping Plan exhibit displays intersection geometrics, bicycle facilities, pedestrian crosswalk striping and lane dimensions. Typical Tee intersection phasing is proposed with a protected northbound left turn phase on N. 7<sup>th</sup> Street. The north side pedestrian crossing phase is proposed to operate with the northbound left turn phase. The northbound contra-flow cycle track movement will be operated with the phase 4 vehicle phase and bicycle head indications will be provided for this movement. Proposed light rail pre-emption is indicated on the plan sheet for both inbound and outbound train traffic. Pre-emption will be accompanied by activated regulatory and warning signs as displayed on the plan. LRT train signal indications will also be provided.



It is proposed to prohibit right turns on red from Bannon Street during the N. 7<sup>th</sup> Street phase 4 green. This is for bicycle safety. It is unlikely that a motorist turning right from Bannon Street would anticipate a northbound bike and a northbound bike may also not be readily visible from the eastbound right turn lane.

The light rail overhead contact system (OCS) will require non-standard poles for mast arms to clear the messenger wires. Exhibit TS-7 displays the equipment dimensions needed for poles on each side of N. 7<sup>th</sup> Street. This considers the existing lines on the east side of N. 7<sup>th</sup> Street as well as new OCS pole locations on the west side.

### **Truck Turn Paths**

WB-50 design vehicle turning movement paths are displayed on the two attached Bannon Street exhibits. Left and right turns into and out of Bannon Street are presented. As shown, a WB-50 design vehicle can satisfactorily negotiate these turns.

## **Pedestrian Crossing Locations**

Crosswalks and associated pedestrian equipment are proposed across Bannon Street and on the north side of the intersection across N. 7<sup>th</sup> Street. A crossing is not proposed at this time on the south side of the intersection due to an existing driveway on the east side of N. 7<sup>th</sup> Street. The driveway location does not permit an acceptable crosswalk alignment. A future extension of Bannon Street to the east will permit installation of an additional crossing on the south side of the intersection.

#### **Bicycle Movements**

Colored bicycle markings are proposed across the west and north legs of the intersection on the inside of the pedestrian crosswalk markings to direct outbound Bannon Street bicycle turning traffic to utilize the pedestrian signals or enter the cycle track. Colored markings will also delineate the cycle track through the intersection. No exclusive bicycle phases are proposed for the intersection.

### **Traffic Signal Equipment**

Placement of the controller and service cabinets is shown at the southwest corner of the intersection located at the rear of the 16' wide sidewalk. It is anticipated that electrical service can be provided in this area and the location of the controller cabinet on this corner will facilitate installation of signal interconnect conduit.

Fiber optic signal interconnect as well as LRT pre-emption cables will be installed on N. 7<sup>th</sup> Street and will connect to existing conduit facilities on the east side of the roadway.



Other equipment will include standard City of Sacramento signal design features such as emergency vehicle pre-emption (GPS based), CCTV PTZ camera installation, modems, video stop bar detection and advance detection on N. 7<sup>th</sup> Street. Equipment details are displayed on exhibits TS-8 through TS-10. Advance detection placement on N. 7<sup>th</sup> Street is shown at 200 feet and is based upon a 35 mph design speed. Video or radar equipment is proposed for the advance detection in place of City standard loops due to the existence of the in-street light rail tracks.

## **Existing Driveways**

An existing driveway is located on the east side of N. 7<sup>th</sup> Street immediately adjacent to the intersection. The driveway is located out of the intersection just south of the proposed northbound stop bar. It is proposed to limit turning movements at this driveway to right turns via signing. Turn prohibition signage will be placed at the back of walk as well as on the signal mast serving southbound N. 7<sup>th</sup> Street.

## N. 7<sup>TH</sup> STREET / N. C STREET INTERSECTION

#### PROJECTED INTERSECTION OPERATIONS

### **Intersection Levels of Service**

Projected intersection operations are summarized in Tables 13 and 14 for baseline plus project and cumulative with project conditions, respectively. As shown, satisfactory LOS C is projected in both the a.m. and p.m. peak hours under both baseline and cumulative plus project conditions. This is within acceptable operating standards identified for the intersection.

TABLE 13 BASELINE PLUS RBOC LEVELS OF SERVICE										
	Acceptable	AM Pea	ak Hour	PM Peak Hour						
Intersection	LOS	LOS	Delay	LOS	Delay					
N. 7 <sup>th</sup> Street / C Street	A - E	С	21	С	26					

TABLE 14 CUMULATIVE PLUS RBOC LEVELS OF SERVICE										
	Acceptable	AM Pea	PM Pea	M Peak Hour						
Intersection	LOS	LOS	Delay	LOS	Delay					
N. 7 <sup>th</sup> Street / C Street	A - E	С	19	С	30					



### **Vehicle Queues**

Tables 15 and 16 summarize projected vehicle queues at the intersection. As shown, projected northbound left turn vehicle queues are projected to exceed the length of the turn pocket and would extend into the number 1 northbound lane. Eastbound queues on N. C Street are projected to extend past the parking garage entrance.

BASELINE P	TABLE 15 BASELINE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES										
AM Peak Hour PM Peak Hour											
Intersection	Storage	Volume	Queue len	Volume	Queue length (feet						
	(feet) (vph) Average 95th %		(vph)	Average	95 <sup>th</sup> %						
N. 7th St / C Street											
Northbound left	60 <sup>1</sup>	98	65	120	42	<25	40				
Eastbound left	250 <sup>2</sup>	37	45	125	234	175	325				
Eastbound right	250	184	200	405	163	195	335				

TABLE 16 CUMULATIVE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES											
AM Peak Hour PM Peak Hour											
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	igth (feet)				
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %				
N. 7th St / C Street											
Northbound left	60 <sup>1</sup>	110	80	130	50	40	100				
Eastbound left	$250^{2}$	35	45	120	275	155	270				
Eastbound right	250	40	55	225	120	150	295				

#### SIGNAL DESIGN SUMMARY

Exhibit TS-5 displays the proposed traffic signal design for the N. C Street / N. 7<sup>th</sup> Street intersection. The attached Striping Plan exhibit displays intersection geometrics, bicycle facilities, pedestrian crosswalk striping and lane dimensions. Typical Tee intersection phasing is proposed with a protected northbound left turn phase on N. 7<sup>th</sup> Street. The northbound contraflow cycle track movement will be operated with the phase 4 vehicle phase and bicycle head indications will be provided for this movement. Proposed light rail pre-emption is indicated on the plan sheet for both inbound and outbound train traffic. Pre-emption will be accompanied by activated regulatory and warning signs as displayed on the plan. LRT train signal indications will also be provided.



It is proposed to prohibit right turns on red from N. C Street during the N. 7<sup>th</sup> Street phase 4 green. This is for bicycle safety. It is unlikely that a motorist turning right from N. C Street would anticipate a northbound bike and a northbound bike may also not be readily visible from the eastbound right turn lane.

The light rail overhead contact system (OCS) will require non-standard poles for mast arms to clear the messenger wires. Exhibit TS-7 displays the equipment dimensions needed for poles on each side of N. 7<sup>th</sup> Street. This considers the existing lines on the east side of N. 7<sup>th</sup> Street as well as new OCS pole locations on the west side.

## **Truck Turn Paths**

Fire truck design vehicle turning movement paths are displayed on the attached N. C Street exhibit. Right turns in and out of N. C Street are presented. As shown, a fire truck design vehicle can satisfactorily negotiate these turns.

### **Pedestrian Crossing Locations**

A crosswalk and associated pedestrian equipment is proposed across N. C Street. No pedestrian facilities are proposed for crossing N. 7<sup>th</sup> Street at this intersection. N. C Street will be a private drive and will primarily provide vehicle access to the on site parking garage and service lot. Pedestrian activity from this area to the east side of N. 7<sup>th</sup> Street is not expected. The existing N. 7<sup>th</sup> Street crossing at N. B Street is located 275' to the south.

### **Bicycle Movements**

No bicycle facilities are proposed for N. C Street. Colored bike markings will delineate the cycle track through the intersection. No exclusive bicycle phases are proposed for the intersection.

#### **Traffic Signal Equipment**

Placement of the controller and service cabinets is shown at the southwest corner of the intersection located at the rear of the 16' wide sidewalk. It is anticipated that electrical service can be provided in this area and the location of the controller cabinet on this corner will facilitate installation of signal interconnect conduit.

Fiber optic signal interconnect as well as LRT pre-emption cables will be installed on N. 7<sup>th</sup> Street and will connect to existing conduit facilities on the east side of the roadway.

Other equipment will include standard City of Sacramento signal design features such as emergency vehicle pre-emption (GPS based), CCTV PTZ camera installation, modems, video stop bar detection and advance detection on N. 7<sup>th</sup> Street. Equipment details are displayed on exhibits TS-8 through TS-10. Advance detection placement on N. 7<sup>th</sup> Street is shown at 200 feet and is based upon a 35 mph design speed. Video or radar equipment is proposed for the advance detection in place of City standard loops due to the existence of the in-street light rail tracks.



## **Existing Driveways**

An existing driveway is located on the east side of N. 7<sup>th</sup> Street and approximately half of the 32' wide driveway will extend into the intersection past the northbound stop bar. This driveway is currently gated and not in use. Property served by this driveway is owned by the City and this property is also served by other driveways. It is proposed to coordinate with the City for the removal of this driveway cut. The driveway cut would be removed in conjunction with construction of the signal system.

## RICHARDS Blvd / N. 6th STREET INTERSECTION

#### PROJECTED INTERSECTION OPERATIONS

#### **Intersection Levels of Service**

Projected intersection operations are summarized in Tables 17 and 18 for baseline plus project and cumulative with project conditions, respectively. As shown, satisfactory LOS C to D is projected in both the a.m. and p.m. peak hours under both baseline and cumulative plus project conditions. This is within acceptable operating standards identified for the intersection.

TABLE 17 BASELINE PLUS RBOC LEVELS OF SERVICE										
	Acceptable	AM Pe	PM Pea	ık Hour						
Intersection	LOS	LOS	Delay	LOS	Delay					
N. 6 <sup>th</sup> Street / Richards Blvd	A – E	D	40	С	29					

TABLE 18 CUMULATIVE PLUS RBOC LEVELS OF SERVICE										
	Acceptable	AM Pe	PM Peak Hour							
Intersection	LOS	LOS	Delay	LOS	Delay					
N. 6 <sup>th</sup> Street / Richards Blvd	A – E	С	21	D	43					

### **Vehicle Queues**

Tables 19 and 20 summarize projected vehicle queues at the intersection. As shown, projected westbound left turn vehicle queues on Richards Blvd can be accommodated within the turn pocket length and center lane area. Northbound queues on N. 6<sup>th</sup> Street can also be accommodated prior to reaching the Bannon Street intersection.



TABLE 19 BASELINE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES											
AM Peak Hour PM Peak Hour											
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	ngth (feet)				
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %				
N. 6th St / Richards Blvd											
Westbound left	$160^{1}$	103	70	130	34	40	135				
Northbound left	4402	78	35	70	336	110	140				
Northbound right	440	35	<25	40	173	140	305				

TABLE 20 CUMULATIVE PLUS RBOC AM/PM PEAK HOUR 95 <sup>TH</sup> PERCENTILE QUEUES											
AM Peak Hour PM Peak Hour											
Intersection	Storage	Volume	Queue len	gth (feet)	Volume	Queue ler	ngth (feet)				
	(feet)	(vph)	Average	95 <sup>th</sup> %	(vph)	Average	95 <sup>th</sup> %				
N. 6th St / Richards Blvd											
Westbound left	160¹	150	115	225	185	105	200				
Northbound left	4402	190	95	140	420	270	440				
Northbound right	440	30	35	105	20	<25	65				

#### **SIGNAL DESIGN SUMMARY**

Exhibit TS-6 displays the proposed traffic signal design for the Richards Blvd / N. 6<sup>th</sup> Street intersection. The attached Striping Plan exhibit displays intersection geometrics, bicycle facilities, pedestrian crosswalk striping and lane dimensions. Typical Tee intersection phasing is proposed with a protected westbound left turn phase on Richards Blvd. Pedestrian phases are proposed across the east side and south side of the intersection.

Raised islands are proposed on the north side of the intersection to fill in the existing passenger drop off area to the LRT station. This drop off area cannot be maintained within the signalized intersection limits. Passenger drop off will be accommodated to the east of the intersection between N. 6<sup>th</sup> and N. 7<sup>th</sup> Streets. The raised islands will also provide transition from the existing north side curb line for westbound traffic, will accommodate traffic signal equipment and will reduce the length of the Richards Blvd pedestrian crossing. The crosswalk length will be reduced from approximately 97' to 75' relative to the existing north side curb location.



## **Truck Turn Paths**

WB-50 design vehicle turning movement paths are displayed on the three attached N. 6<sup>th</sup> Street exhibits. Left and right turns into and out of N. 6<sup>th</sup> Street are presented. As shown, a WB-50 design vehicle can satisfactorily negotiate a westbound left turn from Richards Blvd and a northbound right turn from N. 6<sup>th</sup> Street. Eastbound right turns from Richards Blvd cannot be accommodated without utilizing the full width of N. 6<sup>th</sup> Street. The southwest corner of the intersection will be improved for ADA access within the existing right of way, but this will not permit construction of a curb return to accommodate a WB-50 turn. Future widening of the west side of N. 6<sup>th</sup> Street will mitigate this condition.

In the interim, the project proposes coordinated operation of truck deliveries. Any truck deliveries will generally be within the early morning off-peak hours and these arrivals will be coordinated with the on-site loading dock. It is proposed that northbound N. 6<sup>th</sup> Street will be coned off for a short period of time to accommodate a truck arrival from eastbound Richards Blvd. The limits of this temporary turn area are displayed on the turn exhibit.

## **Pedestrian Crossing Locations**

Crosswalks and associated pedestrian equipment are proposed across N. 6<sup>th</sup> Street and on the east side of the intersection across Richards Blvd. A crossing is not proposed on the west side of the intersection. The majority of all pedestrian crossings of Richards Blvd will be oriented to the southwest corner of the intersection from the RBOC site to the light rail station. An expanded crossing 15' in width is proposed and will include continental bar markings for increased visibility. A west side crossing is not proposed, as RT has requested a parking area for maintenance vehicles on the north side of Richards Blvd to occasionally access the track equipment house on the north side of the light rail lines. Maintenance vehicles currently utilize the existing pull out area. The existing pavement area immediately west of the proposed raised island is intended for this maintenance vehicle parking and this would conflict with a pedestrian crossing on the west side of the intersection.

## **Bicycle Movements**

Colored bicycle markings are proposed across the east and south legs of the intersection on the inside of the pedestrian crosswalk markings to direct bicycle traffic through the intersection. No bicycle facilities are proposed for N. 6<sup>th</sup> Street. Video detection will be provided for the bike lane approaches on Richards Blvd.

#### **Traffic Signal Equipment**

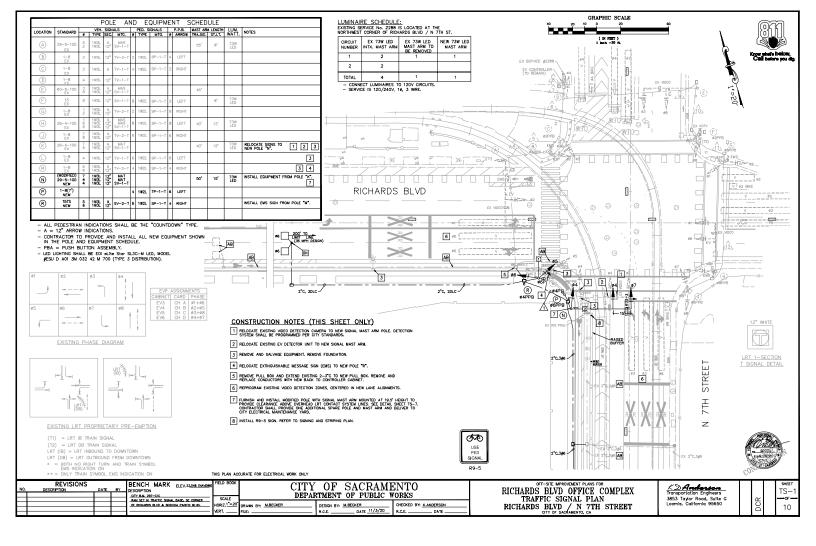
Placement of the controller and service cabinets is shown on the north side of the intersection with the new raised island area. It is anticipated that electrical service can be provided in this area and the location of the controller cabinet on this side will facilitate installation of signal interconnect conduit. The adjacent maintenance vehicle parking area proposed for RT vehicles can then also be utilized by City signal maintenance vehicles.

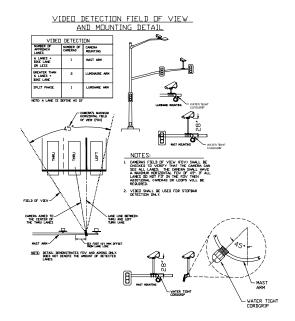


Fiber optic signal interconnect cables will be installed on Richards Blvd and will connect to existing conduit facilities on the north side of the roadway.

Other equipment will include standard City of Sacramento signal design features such as emergency vehicle pre-emption (GPS based), CCTV PTZ camera installation, modems, video stop bar detection and advance detector loops on Richards Blvd. Equipment details are displayed on exhibits TS-8 through TS-10. Advance detection placement on Richards Blvd is shown at 200 feet and is based upon a 35 mph design speed.







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LOOP DET. Ø2	Н	+	+	$\vdash$	+	Н	+	-	н	+	+	Н	+	+	+	+	+	٠	+	2	Н	+	2	_	+	2	-	+	2	+	2	+		H
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#### GENERAL NOTES - TRAFFIC SIGNALS

- GENERAL NOTES TRAFFIC SIGNALS

  TRAFFIC SIGNALS

  TARPIC SOUND, SYSTEM SHALL E CONSTRUCTION SPECIFICATION OF AGRAMMING
  IMPROVIDENT STANDARDS AND CONSTRUCTION SPECIFICATIONS AND THE
  CALTERNS STANDARDS FOR THE STANDARDS STEEDINGNOS, LATEST EDITIONS.

  THIS PLAN ACQUIRATE FOR ELECTRICAL WORK ONLY

  A. ALL LIMPAINES SHALL BE LOS DEFENDED. SHALL BE GALVANIZED.

  5. ALL COMMUNITS SHALL BE SUB-BEDILE 40 PVC UNLESS OTHERWISE SPECIFIED.

  6. ALL REW PVC CORDUIT SHALL ENCONTRAIN 1470 GREEN GROUND CONDUCTOR.

  7. PULL BOXES SHALL BE NO. SO HELDS NOTED OTHERWISE.

  8. INSTALL 6" CONCRETE COLLAR ARCHIO ALL PULL BOXES IN NATIVE SOLL.

  9. THE CONTRACTOR SHALL BE COMPITETED YES REPOSSIBLE FOR CORDINATION
  CIVIL AND STREPHIC WORK WITH ELECTRICAL WORK IN THESE PLANS.

  OF PALL BOXES SHALL BOX IN SOL BECOMES THE SHAPPES.

- CUIL AND STRPING WORK WITH ELECTRICAL WORK IN THESE PLANS.

  10. PALL BORS SAULA FOR EP LACE WITHIN INVESTIGATION FRAMPS.

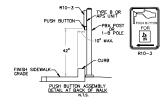
  11. ALL VEHICLE AND PEDESTRAIN INDICATIONS SHALL BE LED.

  12. CONQUICTOR INSULATION SHALL BE THIV UNLESS OTHERWISE NOTED.

  13. TRAFFIC CONTROL SHALL BE PER STRAINARDS AND QUELLES PRESENTED

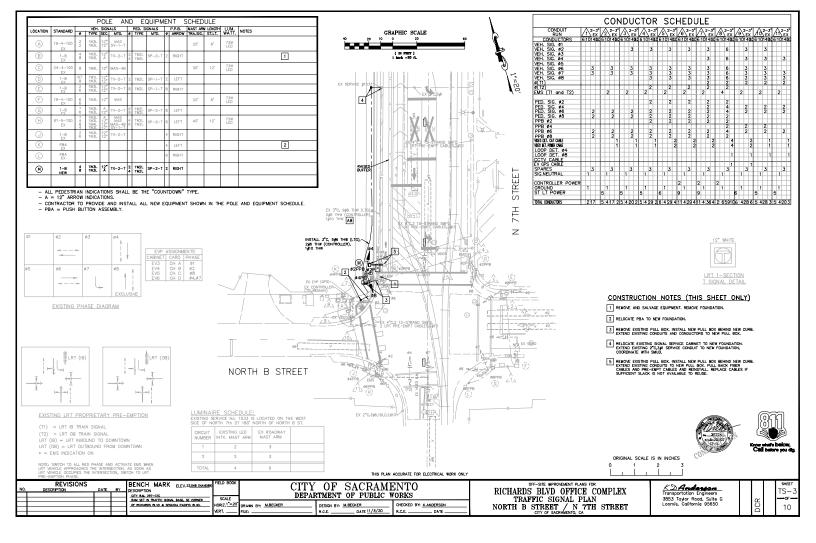
  IN THE CA MUTICO, SHALL BE PER STRAINARDS AND PEDESTRAIN ORGULATION SHALL

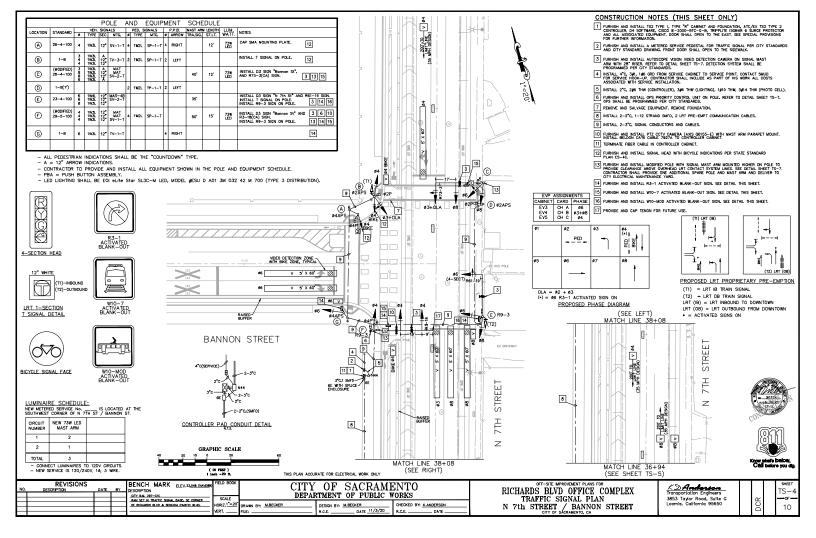
  DE PROVINCE OF ALL THESE.

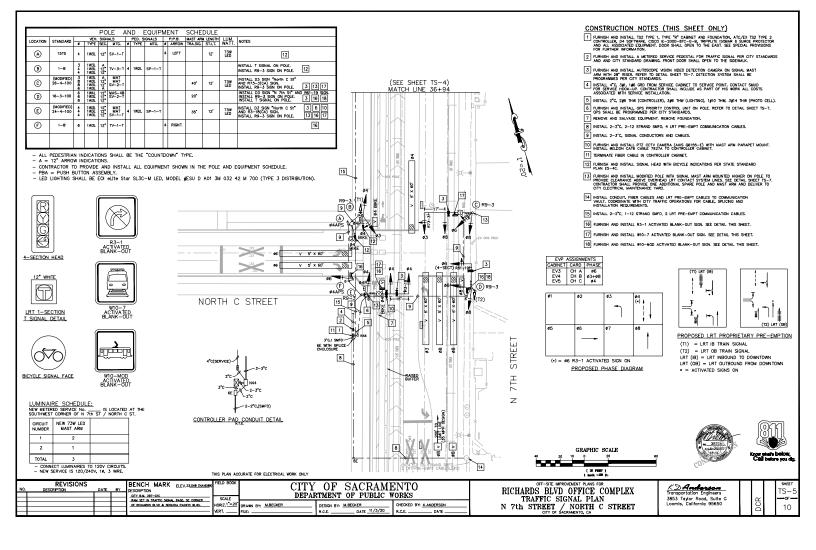


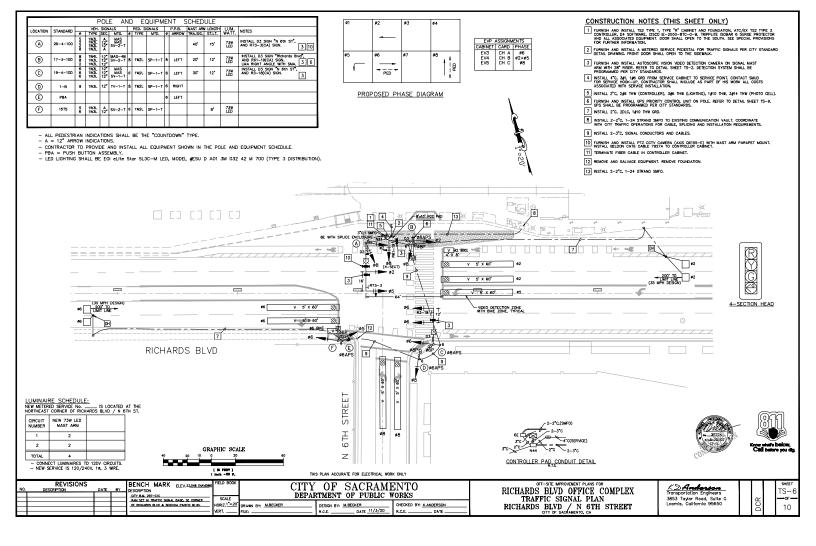


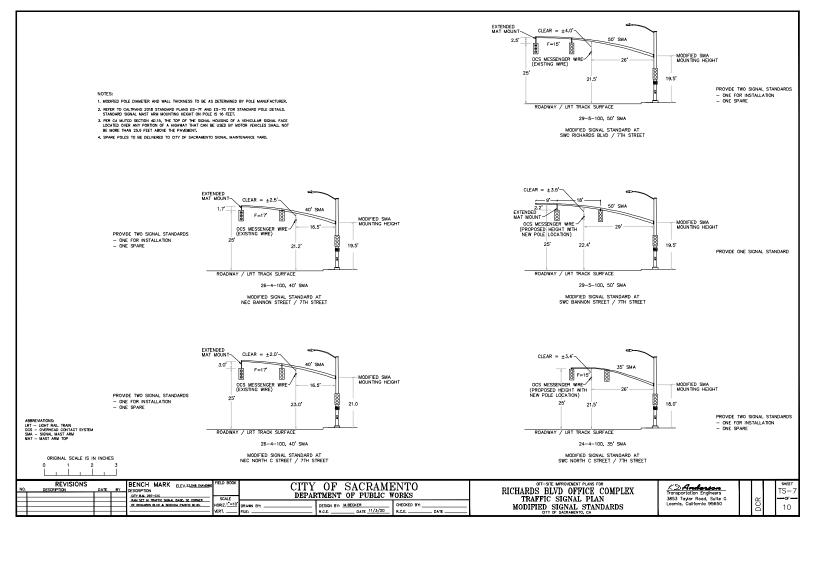
N	DESCRIPTION REVISIONS	DATE BY	BENCH MARK ELEV, 23,046 (NAVDB6) DESCRIPTION GITY BM. 297-C1C RAM SET IN TRAFFIC SIGNAL BASE. SE CORNER	FIELD BOOK	CITY depar	OF SACRAMI	ENTO orks	OFF-SITE IMPROVEMENT PLANS FOR RICHARDS BLVD OFFICE COMPLEX TRAFFIC SIGNAL PLAN	KD Assalance Transportation Engineers 3853 Taylor Road, Suite G	~	TS-:
E			OF MICHARDS MILYO & SEQUIDIA PAGIFIC BLVD.		DRAWN BY: M. BECKER FILE:	DESIGN BY: M.BECKER R.C.E DATE 11/3/20	CHECKED BY: K.ANDERSON R.C.E. DATE	RICHARDS BLVD / N 7TH STREET	Loomis, California 95650	DCF	10

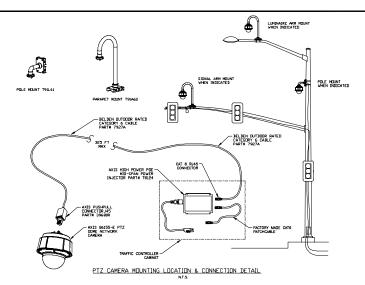












#### CCTV CAMERA SPECIFICATION

- THE CAMERA SYSTEM SHALL HAVE FOLLOWING COMPONENTS OF TRAFFIC OPERATIONS STAFF PRE-APPROVED EQUAL, FOR ADDITIONAL SPECIFICATIONS FOR CCTV SYSTEM, REFER TO THE PROJECT SPECIAL PROVISION.
- FACE TO WITCH 31 MA WAST 1884 HIPE PIDES OFTO HID-SPAN KTUES THE STREET THE SALL HAVE AN HAVE TO PRES OF THE FUNDED 31 MA WAST 1884 HIPE PIDES OFTO HIS-BACKETS SALL HEST THE FILL LICENSME STANDARDS HES BRESSAN, IEEE PER FILECTOR SHALL BE LICENTED IN THE TRAFFIC SIGNAL CABINET. THE PRIMER BLEETOR SHALL BE CONNECTED TO THE RETVIGIN A CATEGORY OF ADMIC CALLE.

- CAMERA MULATING OPTION

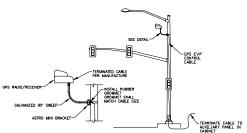
  CAMERA SIANLE, BE MODITED AS INDICATED ON THE PLANS. AXIS POLE MULANT TOPAGE SIANLE BE USED FOR CAMERAS THAT ARE TO BE MULATED ON
  THE VERTICAL SHAPT OF THE SIGNAL POLE AND. AXIS PARAPET MULAT TOPAGE MULATED FOR CAMERA THAT ARE TO BE MULAT ON THE HORIZONTAL
  STOWN, MAST AND
- ALL MOUNTING HARDWARE SHALL BE STAINLESS STEEL. THE CARLE HOLDER IN THE MOUNTING BRACKET SHALL HOLD THE COMMUNICATION CARLE IN PLACE. THE CARLE SHALL HAVE ENDIGH SLACK FOR EASY REMOVAL OF THE CAMERA. ALL HALES BRILLED INTO POLES OR SIGNAL MAST ARMS SHALL HAVE A RUBBER GROWNET TO PROTECT THE COMMUNICATION CARLE.

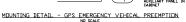
	COMMUNICATION EQUIPMENT FOR R CABINET										
OΤY	DESCRIPTION	PART #	MANUFACTURER	NOTE							
1	CISCO NETWORK SWITCH	IE-2000-8TC-G-8		INSTALL ON DIN RAIL							
1	CISCO POWER SUPPLY	PWR-IE50-AC-IEC	CISCO	INSTALL ON DIN RAIL CONNECT TO SWITCH							
1	SURGE SUPPRESSOR	ISO BAR 6 ULTRA	TRIPP-LITE	INSTALL TO CABINET RAILS							
10	1M CAT 5E PATCH CABLES			INSTALL							
2	SFP MODULE	GLC-LX-SM-RGD	CISCO	INSTALL							
2	1M SMF LC-LC PATCH CORDS			INSTALL							
2	2M SMF LC-LC PATCH CORDS			INSTALL							

- THROUGH A GOOD OLD AUTHORISE PESSLER AND MAINTHET MIST BE NEW AND PROCUPED THROUGH A GOOD OLD AUTHORISED PESSLER AND VALIANCES BY CHARGE OF GOOD AUTHORISED PESSLER AND VALIANCES OF GOOD ACQUANT WANAGER COFFAINBEDGE.CO.COM OF (408) 894–5457. ALL PURCHASES WIST SER REGISTERED WITH GOSO UNDER THE CITY OF SACRAMENTO TARTO ENGINEERING MANE AND COLD IC CITY OF SACRAMENTO—507 (101952802). SMARTTA CACQUANT DOWARD IS MANES AND COLD THROUGH THANKE CITY OF SACRAMENTO TRAFFICE ACCOUNT DOWARD IS AND COLD TO SACRAMENTO TRAFFIC.

CCTV CAMERA EQUIPMENT										
QΤY	DESCRIPTION		MANUFACTURER	NOTES						
1	AXIS NETWORK CCTV WITH POLE INJECTOR	Q6155-E WITH T8124 INJECTOR	AXIS	INSTALL AND TEST						
1	AXIS PARAPET MOUNT	T91A62	AXIS	INSTALL						
50"	BELDON OUTDOOR CAT6	7927A	BELDON	INSTALL						
1	1M CAT6 PATCH CABLES		VARIOUS	INSTALL						

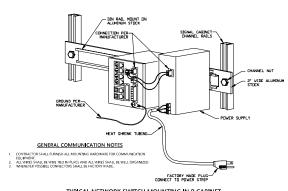
- CONTRACTOR TO PROVIDE ALL EQUIPMENT OR APPROVED EQUAL. APPROVAL SHALL BE BY SIGNAL OPERATIONS STAFF.







-	REVISIONS BENCH MARK FLEY FIELD BOX	* CITY OF CACDAN	IENTO	OFF-SITE IMPROVEMENT PLANS FOR	KD Anderson		SHEET
NO.	DESCRIPTION DATE BY DESCRIPTION SCALE	DEPARTMENT OF PUBLIC WORKS		RICHARDS BLVD OFFICE COMPLEX TRAFFIC SIGNAL PLAN	Transportation Engineers 3853 Taylor Road, Suite G	ı.	TS-8
E		— DRAWN BY: M. BECKER DESIGN BY: M.BECKER  FILE: DATE 11/3/20	CHECKED BY: K.ANDERSON  R.C.E. DATE	TRAFFIC SIGNAL DETAILS CITY OF SACRAMENTO, CA	Loomis, California 95650	DCF	10



TYPICAL NETWORK SWITCH MOUNTING IN R CABINET

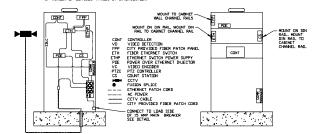
#### GENERAL COMMUNICATION NOTES

L CONTRACTOR IS TO PROVIDE ALL NECESSARY EQUIPMENT IN TRAFFIC CASINET TO PROVIDE COMMUNICATION TO THE TRAFFIC DEPONTING CONTRACTOR FOR COMMUNICATION EQUIPMENT.

3. ALL VIRES SHALL BE VIRE TIED IN PLACE AND ALL VIRES SHALL BE VELL ORGANIZED.

4. ANDREVER POSSIBLE, CONNECTED SHALL BE TATATORY MARK.

5. MARKET D' REVIEES VARIES BY INTERECTION.



TYPICAL IN CABINET COMMUNICATION ROUTING
FOR R CAB WITH FIBER.
NO SCALE CODSTITION SHOWN FOR CLARITYS

TYPICAL IN CABINET COMMUNICATION
MOUNTING FOR R CAB WITH FIBER
NO SCALE

L									
	REVISIONS BENCH MARK ELEV	FIELD BOOK	CITY OF SACRAMENTO DEPARTMENT OF PUBLIC WORKS		RICHARDS BLVD OFFICE COMPLEX TRAFFIC SIGNAL PLAN TRAFFIC SIGNAL PLAN		n n	T	SHEET S-9
F			DRAWN 6Y: M.BECKER   DESIGN BY: M.BECKER   R.C.E.   DATE 11/3/20	CHECKED BY: K.ANDERSON  R.C.E. DATE	TRAFFIC SIGNAL DETAILS CITY OF SACRAMENTO, CA	Loomis, California 95650	DOE		10

FIBER INSTALLATION GENERAL NOTES:

- CONTRACTOR SHALL INSTALL ALL FIBER OPTIC CABLE PER MANUFACTURER'S RECOMMENDATIONS AND THE CITY'S PLANS AND SPECIAL PROVISIONS.
- CONTRACTOR SHALL USE CITY APPROVED CONTRACTOR FOR ALL SPLICE WORK. ALL
  PATCH PANELS AND SPLICE NATURALS WILL BE PROVIDED BY THE CONCRACTOR OR
  CITY APPROVED CONTRACTOR.
- THE CONTRACTOR SHALL TEST ALL FIBER DETIC CARLE SPANS AFTER DISTALATION CONTRACTOR SHALL TEST ALL FIBER DETIC CARLE STALL HEATS THE DISTALATION CHESTS SHALL DEPOSITION CHEST SHALL DEPOSITION CHEST CHEST AND THE MANUFACTURE SECRETARITION AFTER DISTALATION, CITY SHALL MERGET THE MANUFACTURE SHALL NOT SCHEDULES SPLICE WIRK WHITE CARLE IS ACCEPTED BY THE CITY. SPLICE WORK SHALL BE PRETIDENCED IN CITY APPORTED CONTRACTOR.
- THE CONTRACTOR SHALL MAINTAIN A RECORD OF ALL SHATH FOOTAGE MARKENES FOR EACH CABLE SPAN. RECORDS AND A SOFT COPY OF ALL RECORDS SHALL BE PROVIDED TO THE THE STANDARD OF THE PROGRAM TO REVIEW THE SOFT COPY OF THE THE THE STANDARD OF THE PROGRAM TO REVIEW THE SOFT
- 5. EXISTING FIBER OPTIC CABLE MUST BE PROTECTED FROM DAMAGE. THE CONTRACTOR SHALL CONTACT THE CITY INSPECTOR 24 HOURS PRIOR TO WORKING NEAR ANY EXISTING CITY FIBER OPTIC CABLING.
- DURING INSTALLATION THE TENSILE FORCE SHALL NOT EXCEED 600 LBS. THE CONTRACTOR SHALL USE A BREAK AWAY SWIVEL, OR A SLIP-CLUTCH CAPSTAN, SET FOR LESS THEM 600 LBS. PULLING TENSILE STRENGTH.
- THE CONTRACTOR SHALL INSTALL FIBER MARKING TAGS SHALL BE INSTALLED ON FIBER DPTIC CABLE IN EVERY PULL BOX. THE TAGS SHALL BE INSTADHESIVE, HELLERWANN-TYTON 4-FIBER DPTIC MARKER, PART MUNERC NOTED DR APPROVED EQUAL.
- 8. THE CONTRACTOR SHALL INSTALL ARNOD BULL LINE PART WVPIZLC (WITH TRACE WIRE) OR APPROVED EQUAL WITH THE FIBER OPTIC CABLE. FIBER OPTICS CABLE GENERAL NOTES:
- FIBER OPTIC CABLE SHALL BE CORNING ALTDS ALL-DIELECTRIC GEL-FREE OR APPROVED EQUAL, SEE SPECIAL PROVISIONS, APPROVAL SHALL BE BY TRAFFIC OPERATION STAFF.
- CONTRACTOR SHALL INSTALL FIBER PER NANUFACTURE RECONMENDATION
- CONTRACTOR SHALL USE THE FOLLOWING CABLE TYPES AS LISTED BELOW OR APPROVED EQUAL APPROVAL SHALL BE BY THE CITIES TRAFFIC SIGNAL DPERATIONS STAFF.

FIBE	ER OPTIC CABLE	
DESCRIPTION	PART #	MANUFACTURER
12 STRAND	012EU4-T4101D20	CORNING
24 STRAND	024EU4-T4101D20	CERNING
48 STRAND	048EU4-T410LD20	CORNING
96 STRAND	096EU4-T4101D20	CDRNING

FIBER SPLICING/TERMINATION GENERAL NOTES: 1. CONTRACTOR SHALL USE CITY APPROVED FIBER CONTRACTOR FOR ALL FIBER TERMINATION.

- A LIST OF CITY APPROVED FIBER TERMINATION CONTRACTORS CAN BE OBTAIN FORM THE CITY'S INFORMATION TECHNICIOSY DEPARTMENT AT 808-8785 DR FROM TRAFFIC SIGNAL DEPARTMENT GROUP AT 808-6796.
- ALL TERMINATION / SPLICE WORK VILL BE PERFORMED BY CITY APPROVED CONTRACTOR. ALL PATCH PANELS AND SPLICE NATERIALS WILL BE PROVIDED BY CITY APPROVED CONTRACTOR.
- THE CONTRACTOR SHALL CONTACT THE TRAFFIC SIGNAL OPERATIONS GROUP AT 808-6796 FOR FIBER TERMINATION TABLE AND/OR DIAGRAMS.
- THE CONTRACTOR SHALL HAVE CITY APPROVED CONTRACTOR TEST ALL FIBER OPTIC CIRCUITS. TESTING SESUITS MUST BE PROVIDED TO THE CITY INSPECTOR. AFTER FIBER OPTIC CABLE TESTS TEMPOSTRATE THE FISTE OPTIC CABLE STAIL MEDISTRATE HE MANUFACTURES SPECIFICATIONS, THE CABLE AND TERMINATION VORK VILL BE ACCEPTED BY THE CITY.

FIBER SPLICE EQUIPMENT								
DESCRIPTION	PART #	MANUFACTURER	NOTE					
FIBER PATCH PANEL (VALL MOUNT)	VH2121P12-A9-3RH1CD	CORNING	FOR 'R' CABINET					
FIBER PATCH PANEL (VALL MOUNT)	VH2121P12-A9-3RHICD	CORNING	FOR 'G' CABINET					
FIBER PATCH PANEL	PRII2PI2-A9-3RHICD	CORNING	CALTRAN 332/333					
FIBER PATCH PANEL		CORNING	HUBS					
CDYDTE CLDSURE	8006560	PERFORMED LINE	FOR N48 BOX					
CDYDTE CLOSURE SPLICE TRAY	80805514	PERFORMED LINE	FOR N48 BOX					
			FOR 6E BOX					
			FOR 6E BOX					
SMF DUPLEX JUMPERS LC-LC L=2N	0404025131002H	CERNING	SIGNAL CABINET					
SMF DUPLEX JUMPERS LC-LC L=2N	0404025131002M	CORNING	HUB CABINETS					
■ ALL SPLICING EQUIPMENT SHALL BE APPROVED BY TRAFFIC OPERATION STAFF								

#### FIBER OPTICS CABLE TESTING GENERAL NOTES (BARE FIBER TEST):

ALL TESTING SHALL BE PERFERNED ACCIDENING TO THE TELECOMMUNICATIONS HOUSERY ASSOCIATION (TAX TECHNICAL SERVICE BILLETIN TSB-140, ADDITIONAL GUIDELINES FOR FIELD TESTING LENGTH, LOSS AND POLARITY OF OPTICAL FIBER CABLING SYSTEMS.

- THE CONTRACTOR MAY TEST THE FIBER OPTIC CABLE PRIOR TO INSTALLATION, BUT MUST TEST FIBER OPTIC CABLE AFTER INSTALLATION.
- 2. THE CONTRACTOR SHALL TEST EACH FIBER IN THE CABLE.
- 3. A CONTINUITY TEST USING A POWER METER SHALL BE USED FOR 12 STRAND CABLES
  LESS THAN 200 FEET. POWER READING SHALL BE PROVIDED WITH TEST RESULTS.
- 4. THE CONTRACTOR SHALL USE BOTH A POWER NETER TEST FOR CONTINUITY AND AN OPTICA. THE DOMAN REFLECTIONER GOTRO: TO TEST ALL OTHER CARE, SPANS. ORGATER THAN LE STRANGS OR LONGER THAN DO FEET, PROOF OF CALESRATION OF ALL FEBER OPTIC TEST EXUPPIENT MAY BE REQUESTED BY THE INSPECTOR AT ANY THE, AND MIST BE PROVIDED ALDOR WITH THE TEST RESULTS.
- 5. THE FIBER OPTIC CABLE SHALL MEET OR EXCEED ANSI/EIA/TIA 568B3 AND THE LATEST BIGS! TIMM STANDARDS.
- LATES BIGGI TIPM STANDARDS

  FEST RESULTS SHALL DRUGHE THE FILLDUNG

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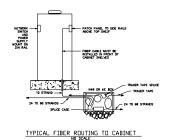
THE CONTRACTOR SHALL PROVIDE THE TEST RESULTS TO THE CITY INSPECTOR. IF FIRST OPTIC CARE, FALS TO RECE CARE, EXTEND STANDARDS AND MR. RIF. CARE AND REPARCH CHILD FOR THE CARE. AND REPARCH CHILD FOR THE CARE. AND REPARCH CHILD FOR THE CARE. THE REPARCH CHILD FOR THE CARE. THE STANDARD CORPORATION TO THESE SHALL BE TESTED ACCORDING TO CARE. THE STANDARD CORPORATION TO THESE SPECIAL PROVISIONS.

#### CONTROLLER COMMUNICATION GENERAL NOTES:

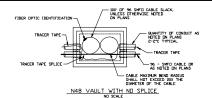
- THE TRAFFIC SIGNAL CONTROLLER SHALL COMMUNICATE VIA ETHERNET INTERFACE MODULE. IF A NETWORK SWITCH OR ETHERNET OVER COPPER DEVICE IS SPECIFIED, THEN THE TRAFFIC CONTROLLER SHALL HAVE ETHERNET INTERFACE MODULE.
- CONTRACTOR SHALL MAKE ALL NECESSARY CONNECTION TO PROVIDE COMMUNICATION TO THE CITY OF SACRAMENTO'S TRAFFIC OPERATION CENTER EXCEPT FOR FIRST SPLICES, RIGHT SPLICES, FIRST SPLICES, SHALL BE COMMETTED BEFORE SIGNAL.

# CITY CONTACTS FOR COMMUNICATION & FIBER OPTIC CABLE: L city of sacramento, traffic operation contex, (966, 809-5067

2. CITY OF SACRAMENTO, DEPARTMENT OF INFORMATION TECHONOLICY, (916) 808-8785



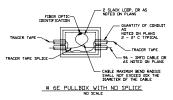


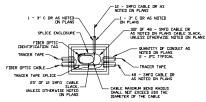


12 - SMFO CABLE OR AS NOTED ON PLANS \_\_L - 3" C DR AS NOTED DN PLANS SPLICE ENCLOSURE -120' DF 96 SMFD CABLE SLACK, UNLESS DTHERWISE NOTED ON PLANS

- TRACER TAPE - 96 - SMFO CABLE OF AS NOTED ON PLANS 25' OF 12 SNFO CABLE SLACK, UNLESS OTHERVISE NOTED ON PLANS CABLE MAXIMUM BEND RADIUS SHALL NOT EXCEED 20X THE DIANETER OF THE CABLE

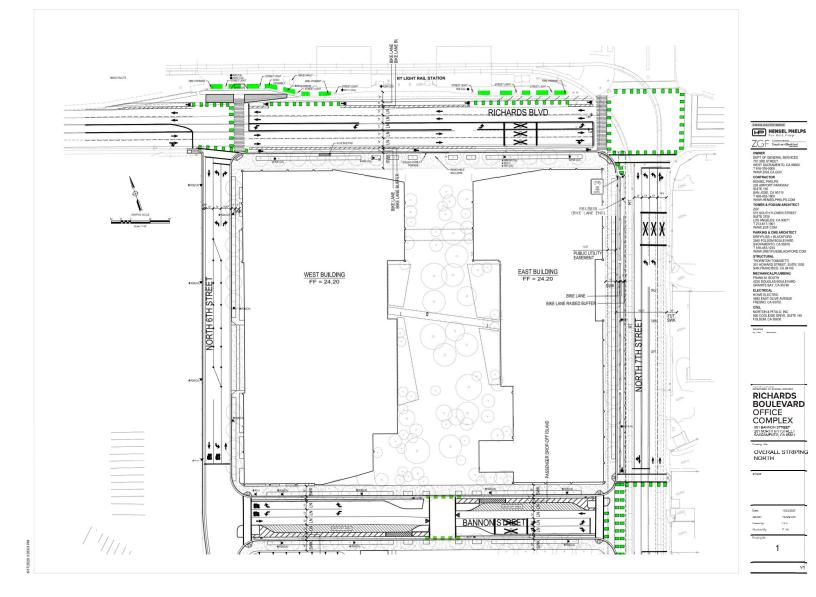
N48 VAULT VITH SPLICE

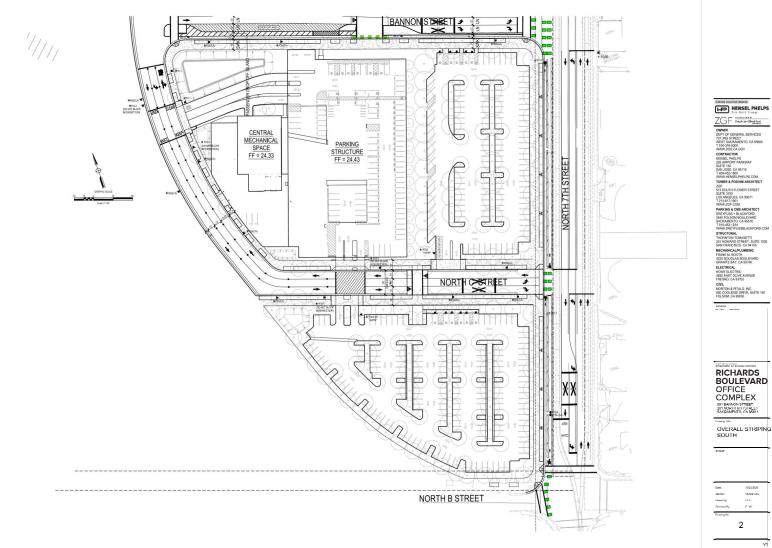


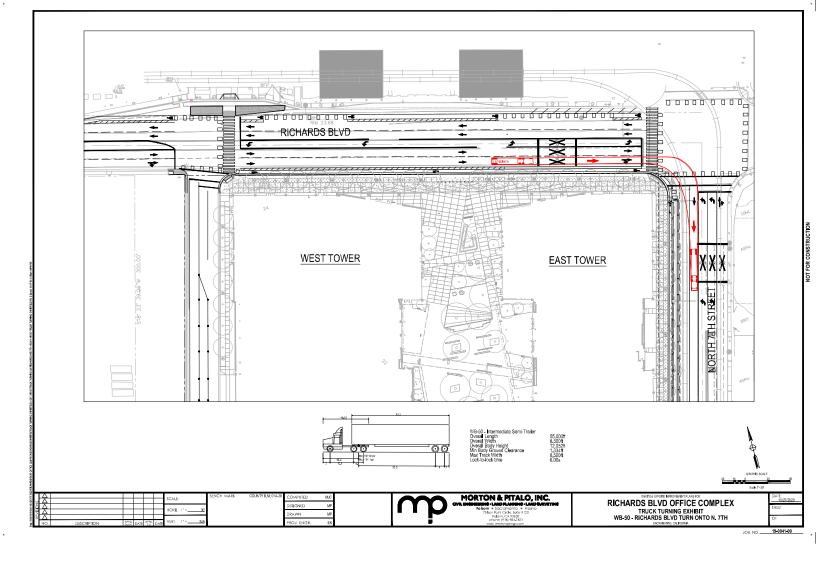


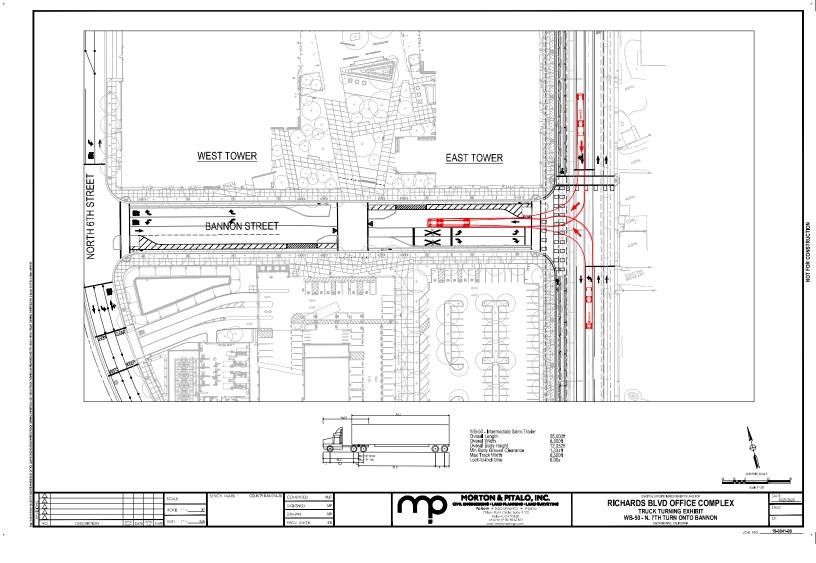
# 6E PULLBOX WITH SPLICE

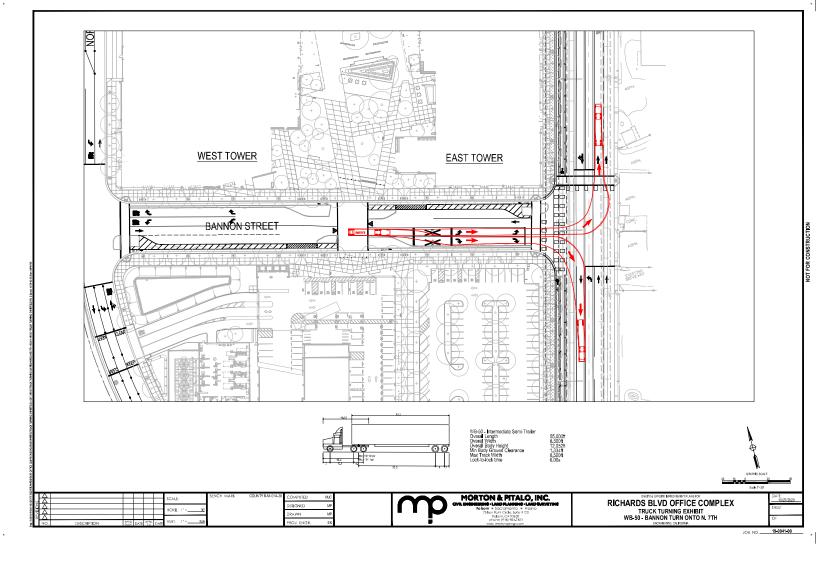
	INFORMATION ON THIS SHIET PROVIDED BY THE CITY OF SACKAMENTO											
NO.	REVISIONS DESCRIPTION	DATE BY	BENCH MARK ELEV	FIELD BOOK	CITY	OF SACRAMENTO MENT OF PUBLIC WORKS		OFF-SITE MAPROVEMENT PLANS FOR RICHARDS BLVD OFFICE COMPLEX TRAFFIC SIGNAL PLAN	Transportation Engineers 3853 Taylor Road, Suite G		$\prod$	SHEET TS-10
				HORIZ. NON VERT.		DESIGN BY: M.BECKER R.C.E DATE	CHECKED BY: <u>K.ANDERSON</u> R.C.E. DATE	TRAFFIC SIGNAL DETAILS	Loomis, California 95650	5	2	10

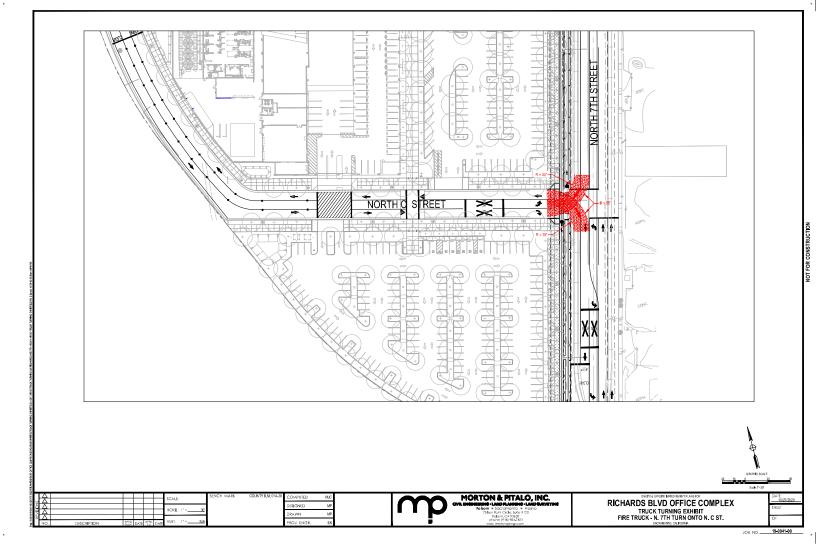


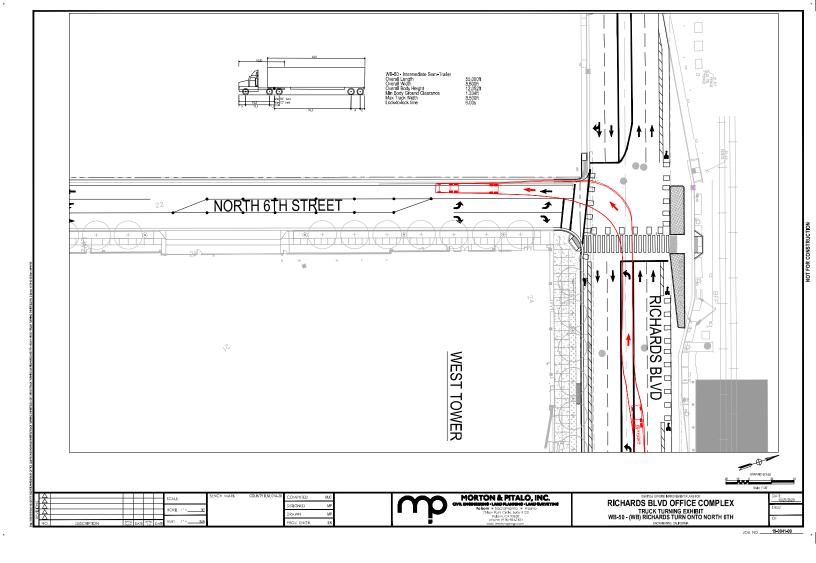


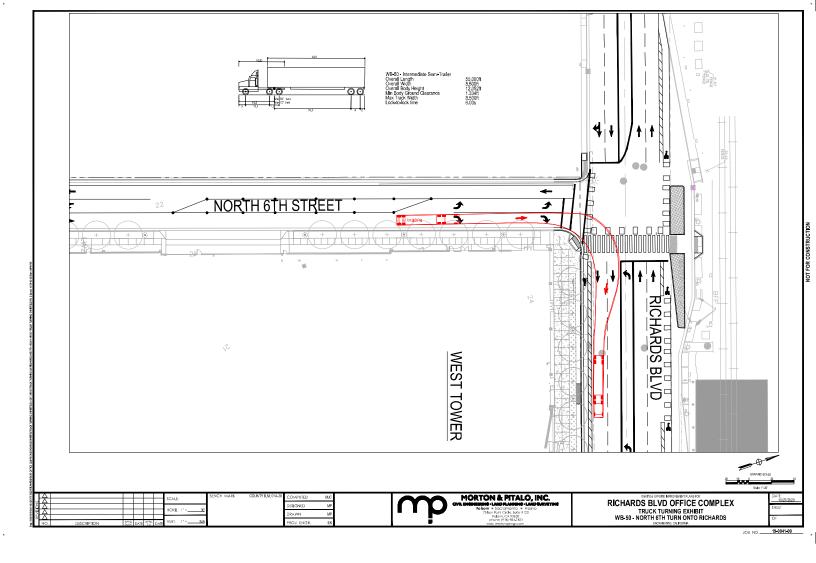


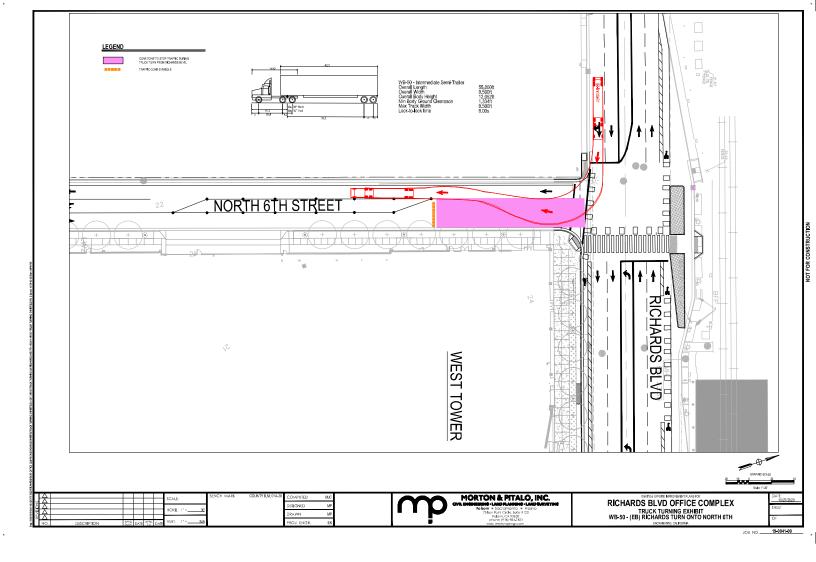












## Appendix

Traffic Signal Preemption Technical Memorandum, Kimley Horn



# Memorandum

To: Kenneth Anderson, P.E. and Mike Becker, P.E.

KD Anderson & Associates, Inc.

From: Robert Paderna, P.E.

Kevin Aguigui, P.E., T.E., E.E., CSEP

Re: DGS Richards Boulevard - Traffic Signal Preemption Technical Memorandum.

Date: November 3, 2020

#### INTRODUCTION

This memorandum documents the light rail preemption operations for two proposed traffic signal installations to be constructed as part of the Richards Boulevard Office Complex (RBOC) project (the "project"). These two new traffic signals, which are to be located at the intersections of North 7<sup>th</sup> Street/Bannon Street and North 7<sup>th</sup> Street/North C Street (Attachment A), are proposed to provide site access the proposed redevelopment of the site located at the southwest quadrant of the Richards Boulevard/North 7<sup>th</sup> Street intersection into a Department of General Services (DGS) office building complex. As required by the City of Sacramento, a Design Concept Report (DCR) is being prepared by KD Anderson & Associates, Inc. for the new traffic signals and existing traffic signals to be modified as part of the project. The intent of this memorandum is to provide input on the light rail operational parameters for consideration in the DCR.

The Sacramento Regional Transit District (RT) currently operates the Green Line light rail along North 7<sup>th</sup> Street, with the northern terminus at the 7<sup>th</sup> & Richards/Township 9 LRT station located at the west leg of the Richards Boulevard/North 7<sup>th</sup> Street intersection. The Green Line LRT is in-street running along the segment of North 7<sup>th</sup> Street within the project area, with "Inbound" running in the southbound travel lane and "Outbound" running in the northbound travel lane. This memorandum presents recommended preemption operational strategy and timing for the interface between the LRT trains and the two new City-owned and maintained traffic signals.

### **EXISTING OPERATIONS**

LRT train detection along the Green Line is currently provided by a combination of existing train to wayside communication (TWC) loop detectors, track circuits, and wheel detectors. These detection strategies are also used for traffic signal preemption. The interface between the LRT and the City traffic signal controllers are provided via existing preemption cables running along North 7<sup>th</sup> Street. This communication path between LRT detectors and the traffic signal controllers is critical to providing and maintaining preemption capabilities to optimize system performance and reduce delays at the intersections. **Attachment B** provides track plans which present the existing detector locations and distances used in development of the recommended preemption timing.

As an LRT train approaches a signalized intersection and a train is detected, a call is placed to the downstream traffic signal controller(s) to initiate the preemption phase. This would initiate the clear out

phase in which vehicular phases would be terminated; however, the controllers are not configured to truncate conflicting pedestrian phases, allowing the full pedestrian phase to be served. Delay timing, if entered, would delay the preempt timer in the traffic signal controller when the approach time (calculated based on 25 mph train speed) is higher than the required signal timing of the clear out phase. Once the delay timer expires, signal will go into the clearout phase. Once the clear out phase is complete, the designated phases in conflict with the LRT phase transitions to yellow and red before the point in which the LRT track signals are activated to indicate that it is clear for the LRT train to enter the intersection. Once the train passes the designated release detector, the track signal transitions to red and the traffic signal returns to normal operation once sufficient time has elapsed for the rear of the train to clear the intersection.

### PREEMPTION BY INTERSECTION

#### North 7th Street/Bannon Street

The new intersection at North 7<sup>th</sup> Street/Bannon Street is proposed to be a "T-intersection" providing primary access to the RBOC site from North 7<sup>th</sup> Street. The proposed traffic signal will provide a protected northbound left-turn phase onto Bannon Street. Pedestrian phases will be provided crossing Bannon Street (west leg) and crossing North 7<sup>th</sup> Street (north leg). LRT preemption will be provided for inbound (southbound) and outbound (northbound) trains which are in-street running as summarized below.

#### Inbound

Trains departing the 7<sup>th</sup> & Richards/Township 9 LRT station passes over A141TWC loop detector and sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/Bannon Street intersection from the Richards Boulevard/North 7<sup>th</sup> Street controller via preempt cable. Secondary detection is provided when the train passes over A137AWD wheel detector which sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/Bannon Street intersection also from the Richards Boulevard/North 7<sup>th</sup> Street controller.

The traffic signal controller will terminate conflicting vehicular phases and track signal is activated once the pedestrian phase crossing North 7<sup>th</sup> Street (north leg) ends. Only northbound thru vehicular phase (phase 8) will be permitted concurrent with track signal activation. Additionally, pedestrian phase 4 will be permitted concurrent with track signal activation.

Refer to traffic signal plans (sheet TS-4) provided in **Attachment C** for the proposed phase diagram and preemption diagram. LRT timing and operational parameters are provided in **Attachment D**.

Required Signal Timing Clearance – 32 seconds

Approach Time -31 seconds at 25 mph

**Delay Timing –** 0 seconds

**Release** – As the train enters the intersection, it passes A122AWD wheel detector and sends a release to the traffic signal controller. This will terminate the preempt phase once sufficient time has elapsed for the rear of the train to clear the intersection.

#### Outbound

Trains passes over A070BWD wheel detector and sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/Bannon Street intersection from the "house" at UPRR underpass south of North B Street via preempt cable. Secondary detection is provided when the train sends an output from A094RC which sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/Bannon Street intersection from the North 7<sup>th</sup> Street/North B Street controller.

The traffic signal controller will terminate conflicting vehicular phases and track signal is activated once the pedestrian phase crossing North 7<sup>th</sup> Street (north leg) ends. Only northbound (phases 3 and 8) or southbound (phase 4) vehicular phases will be permitted concurrent with track signal activation. Additionally, pedestrian phase 4 will be permitted concurrent with track signal activation.

Refer to traffic signal plans (sheet TS-4) provided in **Attachment C** for the proposed phase diagram and preemption diagram. LRT timing and operational parameters are provided in **Attachment D**.

Required Signal Timing Clearance – 32 seconds

**Approach Time** – 78 seconds at 25 mph

Delay Timing – 46 seconds

**Release** – As the train enters the intersection, it passes A122BWD wheel detector and sends a release to the traffic signal controller. This will terminate the preempt phase once sufficient time has elapsed for the rear of the train to clear the intersection.

#### North 7th Street/North C Street

The new intersection at North 7<sup>th</sup> Street/North C Street is proposed to be a "T-intersection" providing secondary access to the RBOC site from North 7<sup>th</sup> Street. The proposed traffic signal will provide a protected northbound left-turn phase onto North C Street. A pedestrian phase will be provided crossing North C Street (west leg) only; no pedestrian phase is proposed crossing North 7<sup>th</sup> Street at this intersection. LRT preemption will be provided for inbound (southbound) and outbound (northbound) trains which are in-street running as summarized below.

#### Inbound

Trains departing the  $7^{th}$  & Richards/Township 9 LRT station passes over A137AWD wheel detector and sends a call to the traffic signal controller at the North  $7^{th}$  Street/North C Street intersection from the Richards Boulevard/North  $7^{th}$  Street controller via preempt cable. Secondary detection

is provided when the train passes over A122AWD wheel detector which sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/North C Street intersection from the North 7<sup>th</sup> Street/Bannon Street controller.

The traffic signal controller will terminate conflicting vehicular phases and track signal is activated. Only northbound thru vehicular phase (phase 8) will be permitted concurrent with track signal activation. Additionally, pedestrian phase 4 will be permitted concurrent with track signal activation.

Refer to traffic signal plans (sheet TS-5) provided in **Attachment B** for the proposed phase diagram and preemption diagram. LRT timing and operational parameters are provided in **Attachment C**.

**Required Signal Timing Clearance** – 32 seconds

**Approach Time** – 31 seconds at 25 mph

**Delay Timing** – 0 seconds

**Release** – As the train enters the intersection, it sends a release signal to the traffic signal controller. This will terminate the preempt phase once sufficient time has elapsed for the rear of the train to clear the intersection.

#### Outbound

Trains passes over A070BWD wheel detector and sends a call to the traffic signal controller at the North 7<sup>th</sup> Street/North C Street intersection from the "house" at UPRR underpass south of North B Street via preempt cable.

The traffic signal controller will terminate conflicting vehicular phases and track signal is activated. Only northbound (phases 3 and 8) or southbound (phase 4) vehicular phases will be permitted concurrent with track signal activation. Additionally, pedestrian phase 4 will be permitted concurrent with track signal activation.

Refer to traffic signal plans (sheet TS-5) provided in **Attachment C** for the proposed phase diagram and preemption diagram. LRT timing and operational parameters are provided in **Attachment D**.

Required Signal Timing Clearance – 32 seconds

**Approach Time** – 67 seconds at 25 mph

**Delay Timing –** 35 seconds

**Release** – As the train enters the intersection, it passes A122BWD wheel detector at North 7<sup>th</sup> Street/Bannon Street and sends a release to the traffic signal controller. This will terminate the preempt phase.

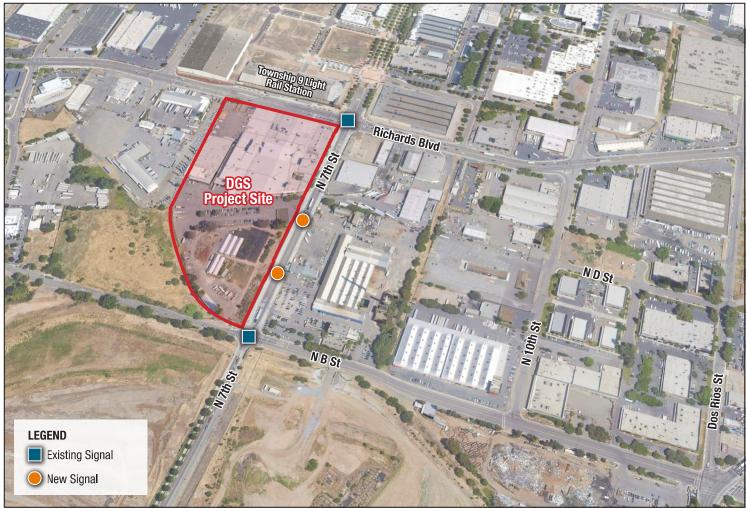
## Attachments:

Attachment A – Project Area Map

Attachment B – Track Plans (prepared by LTK Engineering)

Attachment C – Traffic Signal Plans (prepared by KD Anderson)

Attachment D – LRT Preempt Summary Tables



Source: Google Maps, 2020

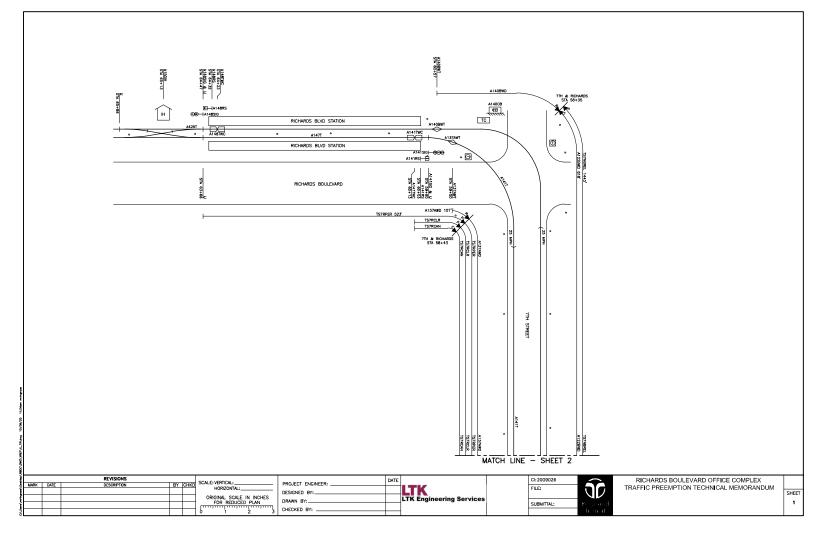
**ATTACHMENT A:** Project Area Map DGS RICHARDS BOULEVARD - TRAFFIC SIGNAL PREEMPTION City of Sacramento

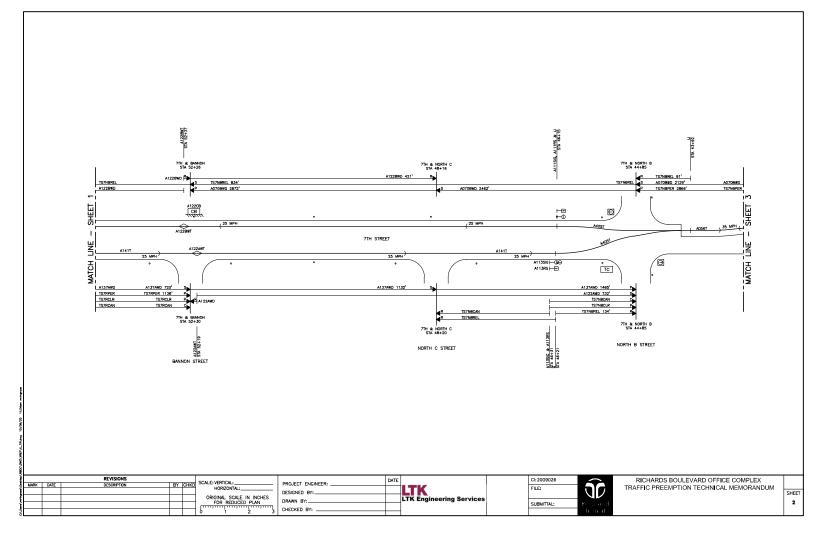


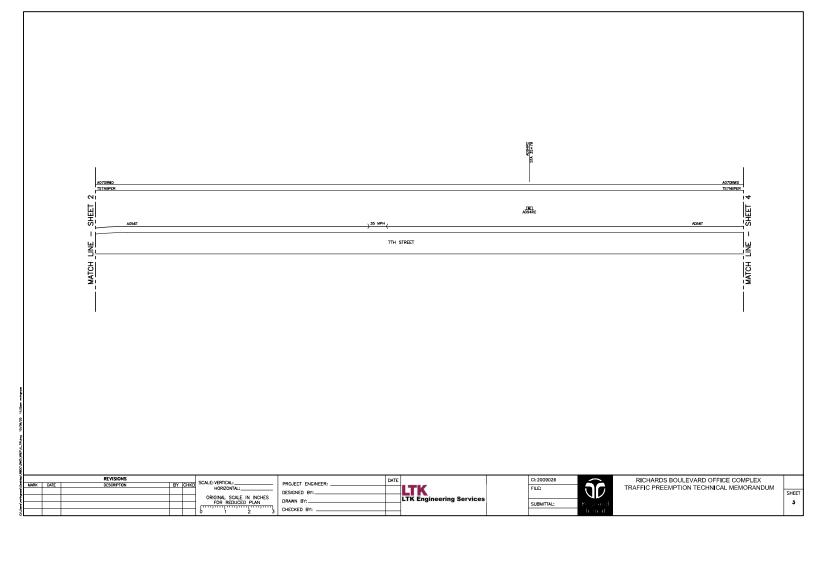


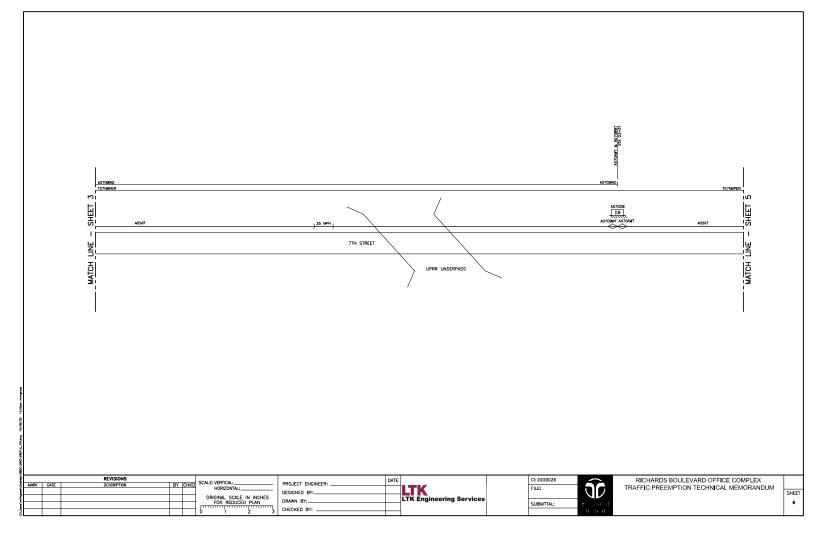


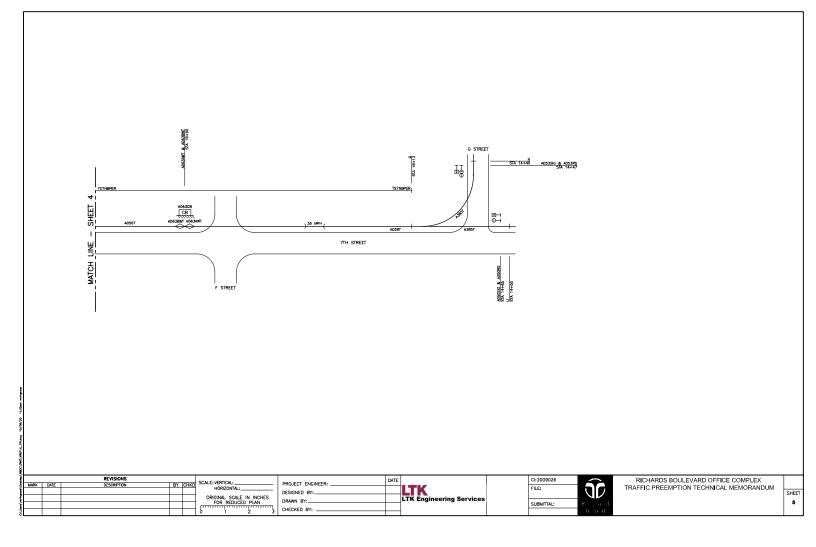
Attachment B – Track Plans (prepared by LTK Engineering)

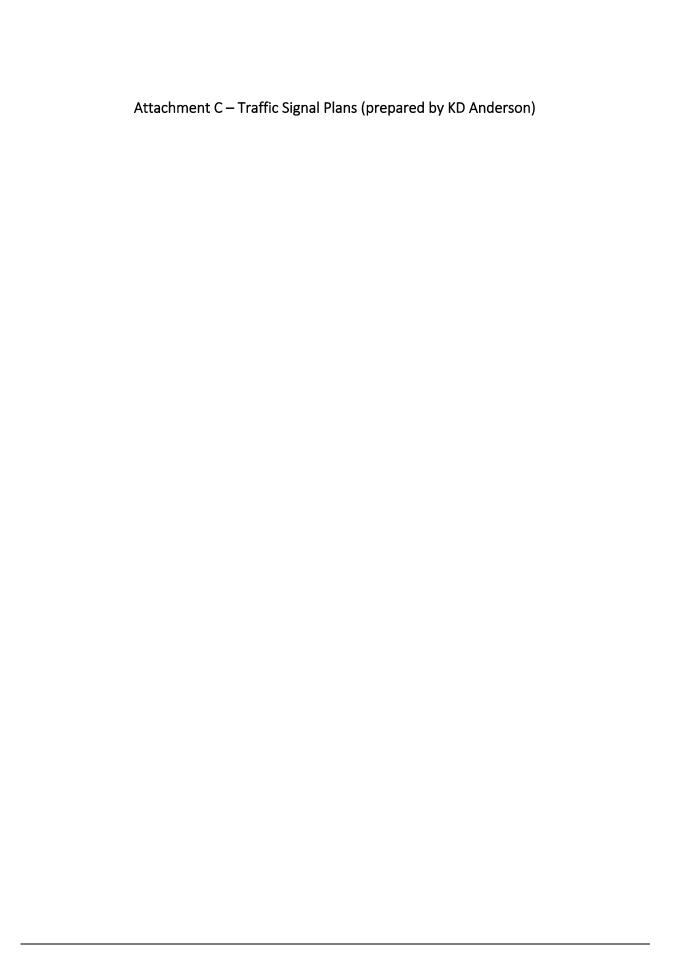


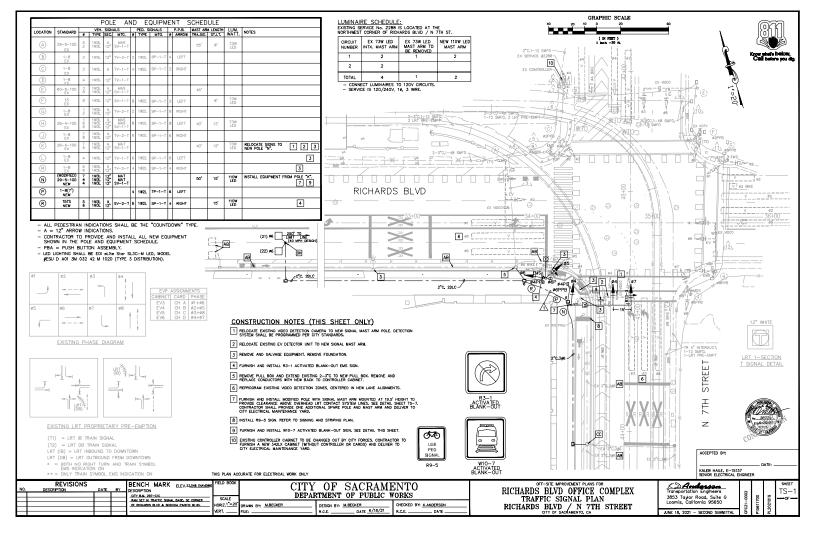


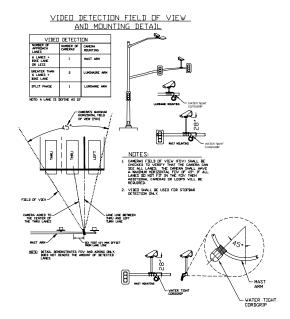












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#### GENERAL NOTES - TRAFFIC SIGNALS

- GENERAL NOTES TRAFFIC SIGNALS

  TRAFFIC SIGNALS

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  A. ALL EVEN TRAFFIC SIGNAL STANDARDS SHALL BE GALVANIZZD.

  A. ALL DEW PLOT SHALL BE SOMEDIALE OF PUT UNLESS OTHERWISE SPECIFIED.

  A. ALL REW PUT CONDUIT SHALL ECONTAIN 1470 GREEN GROUND CONDUCTOR.

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- CUIL AND STRPING WORK WITH ELECTRICAL WORK IN THESE PLANS.

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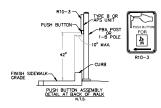
  11. ALL VEHICLE AND PEDESTRAIN INDICATIONS SHALL BE LED.

  12. CONQUICTOR INSULATION SHALL BE THIV UNLESS OTHERWISE NOTED.

  13. TRAFFIC CONTROL SHALL BE PER STRAINARDS AND QUELLES PRESENTED

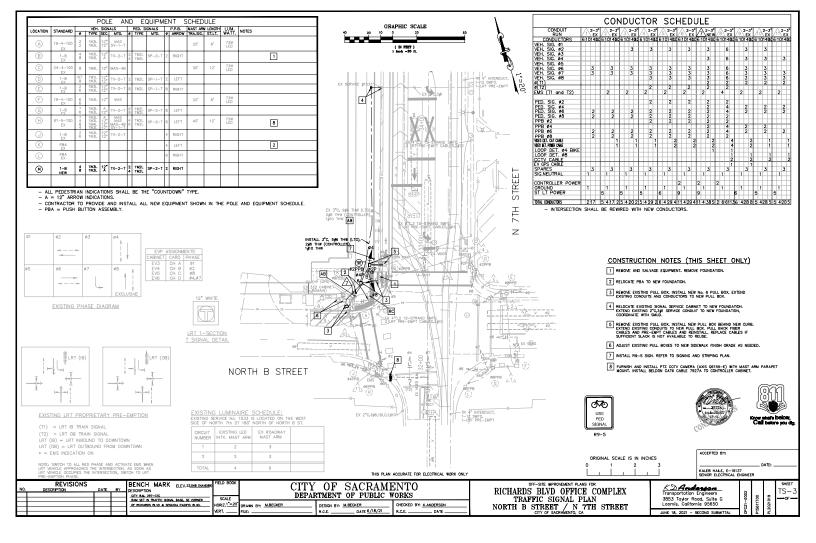
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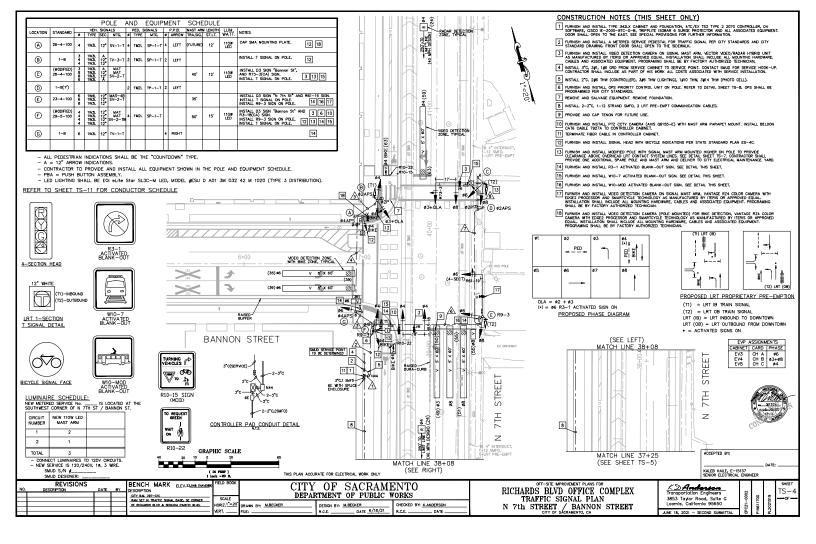
  DE PROVINCE OF ALL THESE.

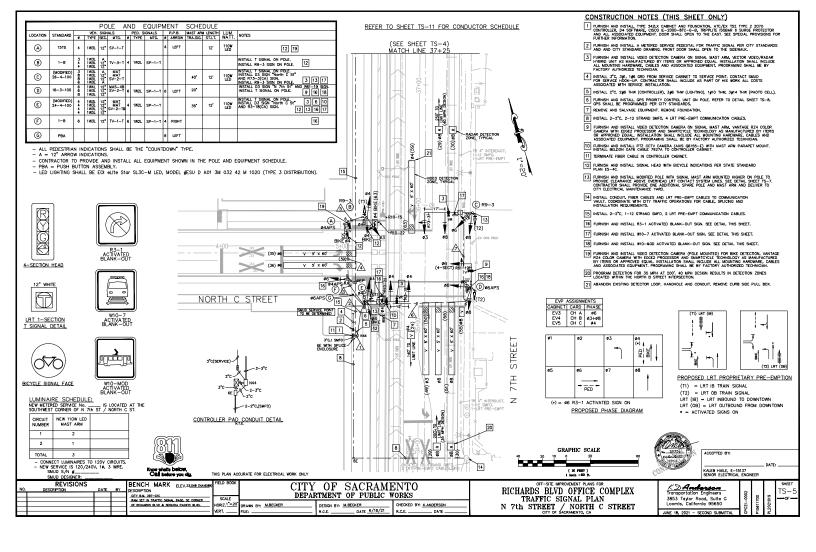




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NO.	REVISIONS DESCRIPTION	DATE B	Y BE	ENCH MARK ELEV, 23 <u>,048 (NAVD88)</u> SCRIPTION	FIELD BOOK	l CITY	OF SAUKAN		OFF-SITE IMPROVEMENT PLANS FOR RICHARDS BLVD OFFICE COMPLEX	KD Anderson			SHEET TS = 2
			RA	TY B.M. 297-CIC NA SET IN TRAFFIC SIGNAL BASE, SE CORNER	SCALE		TMENT OF PUBLIC		TRAFFIC SIGNAL PLAN	Transportation Engineers 3853 Taylor Road, Suite G Loomis, California 95650	2000	1700	—or—
E					HORIZ VERT	DRAWN BY: M. BECKER FILE:	DESIGN BY: M.BECKER R.C.E. DATE 6/18/21	R.C.E DATE	RICHARDS BLVD / N 7TH STREET	JUNE 18, 2021 - SECOND SUBMITTAL	OP C21	P1561	









#### Richards Blvd OC LRT Preempt

					Timing						
INTERSECTION LOCATION	TRAIN DETECTOR FUNCTION	RT SIGNAL DETECTOR NAME	RT SIGNAL DETECTOR LOCATION	REQUIRED SIGNAL TIMING (SEC) *	APPROACH TIME (SEC)	DELAY TIMING (SEC)	CONNECTIONS BETWEEN RT'S CB AND CITY CONTROLLERS	7th&Bannon T-Signal Activation and Release	7th&Bannon	Pedestrian Crossing Signals 7th&Bannon Activation and Aspect	COMMENTS
	Primary	A141TWC Loop or TS7RPER	Richards Blvd Station	32 sec (7 sec Walk + 20 sec FDW +	31	0	City wiring between the existing 7th & Richards City Controller and the RT CB	T-SIGNAL SHOULD BE ACTIVATED ONCE CROSS STREET (XING	TERMINATE CONFLICTING VEHICLE PHASES AFTER DELAY. IF CONFLICTING PED PHASE IS SERVED, CONTINUE SERVING UNTIL IT IS	PEDESTRIAN SIGNAL CROSSING THE WEST	
N 7th & Bannon St (INBOUND)	Secondary	A137AWD Wheel Detector	7th & Richards	20 sec FDW + 4 sec Yellow + 1 sec All-Red)	20	0	City wiring between the existing 7th & Richards City Controller and the RT CB	7TH ST) PEDESTRIAN PHASE ENDS.	COMPLETED. T- SIGNAL WILL BE ACTIVATED. ONLY NB THRU PHASE WILL BE GREEN CONCURRENT WITH T-SIGNAL.	LEG WILL HAVE "WALK" SIGNAL	
	Release	A122AWD Whee <b>l</b> Detector	7th & Bannon		N/A	N/A	City wiring between the new 7th & Bannon City Controller and the RT CB	T-SIGNAL IS RELEASED AND NORMAL TRAFFIC CONTROL RESUMES	EB PHASE WILL BECOME "GREEN" AS TRAIN CLEARS.	PEDESTRIAN SIGNAL CROSSING THE NORTH LEG WILL HAVE "WALK" SIGNAL	
	Primary	A070BWD Wheel Detector	UPRR UNDERPASS	32 sec (7 sec Walk + 20 sec FDW +	78	46 sec	City wiring between the existing 7th & B City Controller and the RT CB	T-SIGNAL SHOULD BE ACTIVATED ONCE CROSS STREET (XING THIST) PEDESTRIAN PHASE ENDS.	TERMINATE CONFLICTING VEHICLE PHASES AFTER DELAY. IF CONFLICTING PED PHASE IS SERVEN CONTINUE SERVING UNTIL IT IS COMPLETED. T- SIGNAL WILL BE ACTIVATED. NB/SB THRU PHASES WILL BE GREEN CONCURRENT WITH T-SIGNAL.	PEDESTRIAN SIGNAL CROSSING THE WEST LEG WILL HAVE "WALK" SIGNAL	
N 7th & Bannon St (OUTBOUND)	Secondary	TS7NBREL	7th & N B	4 sec Yellow + 1 sec All-Red)	23	0	City wiring between the existing 7th & B City Controller and the RT CB				
	Release	A122BWD Whee <b>l</b> Detector	7th & Bannon		N/A	N/A	City wiring between the new 7th & Bannon City Controller and the RT CB	T-SIGNAL IS RELEASED AND NORMAL TRAFFIC CONTROL RESUMES	EB PHASE WILL BECOME "GREEN" AS TRAIN CLEARS.	PEDESTRIAN SIGNAL CROSSING THE NORTH LEG WILL HAVE "WALK" SIGNAL	

<sup>\*</sup> Walk+FDW+Yellow+All Red time

TS7RPER: Existing RT output from A150IH delivered to 7th & Richards TC. (Track circuit A147T is occupied and no A141SIG route has been called and the previous A141SIG route had not been cancelled.)
TS7RCAN: Existing RT output from A150IH delivered to 7th & Richards TC. (Track circuit A147T is occupied and A141SIG has been cleared and then cancelled.)
TS7RCAR: Existing RT output from A150IH delivered to 7th & Richards TC. (Track circuit A147T is occupied and an A141SIG route has been cleared and the previous A141SIG route had been cancelled.)
A137AWD: Existing RT output from A140CB delivered to 7th & Richards TC.
A122AWD: Existing RT output from A170CB delivered to 7th & Richards TC.
A175NPREIL: Existing RT output from A047CB delivered to 7th & North B TC.
TS7NPREIL: Existing RT output from A047CB delivered to 7th & North B TC.
TS7NPREIL: Existing RT output from A047CB delivered to 7th & Richards TC.

#### Richards Blvd OC LRT Preempt

	TDAIN	DT GIONAL	DT GIONAL		Timing						
INTERSECTION LOCATION	TRAIN DETECTOR FUNCTION	RT SIGNAL DETECTOR NAME	RT SIGNAL DETECTOR LOCATION	REQUIRED SIGNAL TIMING (SEC) *	APPROACH TIMING (SEC)		CONNECTIONS BETWEEN RT'S CB AND CITY CONTROLLERS	7th&C T-Signal Activation and Release	Traffic Signals 7th&C. Activation and Aspect	Pedestrian Crossing Signals 7th&C. Activation and Aspect	COMMENTS
	Primary	A137AWD Wheel Detector	7th & Richards	32 sec	31	0	City wiring between the existing 7th & Richards City Controller and the RT CB	T-SIGNAL MAY BE ACTIVATED ANYTIME	TERMINATE CONFLICTING VEHICLE PHASES AFTER DELAY. T-	PEDESTRIAN SIGNAL CROSSING THE WEST	
N 7th & C (INBOUND)	Secondary	A122AWD Wheel Detector	7th & Bannon	split)	11	0	City wiring between the new 7th & Bannon City Controller and the RT CB	(NO CONFLICTING PED PHASE)	SIGNAL WILL BE ACTIVATED. ONLY NB THRU PHASE WILL BE GREEN CONCURRENT WITH T-SIGNAL.	LEG WILL HAVE "WALK" SIGNAL	
	Release	TS7NBREL / TS7NBCAN	7th & N B		N/A	N/A	City wiring between the existing 7th & B City Controller and the RT CB	T-SIGNAL IS RELEASED AND NORMAL TRAFFIC CONTROL RESUMES	EB PHASE WILL BECOME "GREEN" AS TRAIN CLEARS.	N/A	
	Primary	A070BWD Wheel Detector	UPRR UNDERPASS	32 sec	67	35 sec	City wiring between the existing 7th & B City Controller and the RT CB	T-SIGNAL MAY BE ACTIVATED ANYTIME	TERMINATE CONFLICTING VEHICLE PHASES AFTER DELAY. T- SIGNAL WILL BE	PEDESTRIAN SIGNAL CROSSING THE WES	
N 7th & C (OUTBOUND)	Secondary			(assumed min split)				(NO CONFLICTING PED PHASE)	ACTIVATED. NB/SB THRU PHASES WILL BE GREEN CONCURRENT WITH T- SIGNAL.	LEG WILL HAVE "WALK" SIGNAL	
	Release	A122BWD Wheel Detector	7th & Bannon		N/A	N/A	City wiring between the new 7th & Bannon City Controller and the RT CB	T-SIGNAL IS RELEASED AND NORMAL TRAFFIC CONTROL RESUMES	EB PHASE WILL BECOME "GREEN" AS TRAIN CLEARS.	N/A	

<sup>\*</sup> Walk+FDW+Yellow+All Red time

A137AWD: Existing RT output from A140CB delivered to 7th & Richards TC,
TS7NBREL: Existing RT output from A094RC delivered to 7th & North B TC. (Track circuit A405T is occupied. Expires after 10 seconds.)
TS7NBCAN: Existing RT output from A094RC delivered to 7th & North B TC. (Either A111SIG or A113SIG is cancelled after a route had been requested.)
A070BWD: Existing RT output from A070CB delivered to 7th & North B TC.
A122BWD: Existing RT output from A122CB delivered to 7th & Richards TC.